Project:

VILLA VICTORIA CENTER FOR THE ARTS 85 WEST NEWTON STREET, BOSTON, MA

PHASE 1A CONSTRUCTION DOCUMENTS 9/22/2017

Prepared For:

INQUILINOS BORICUAS EN ACCION

405 Shawmut Ave, Boston, MA 02118

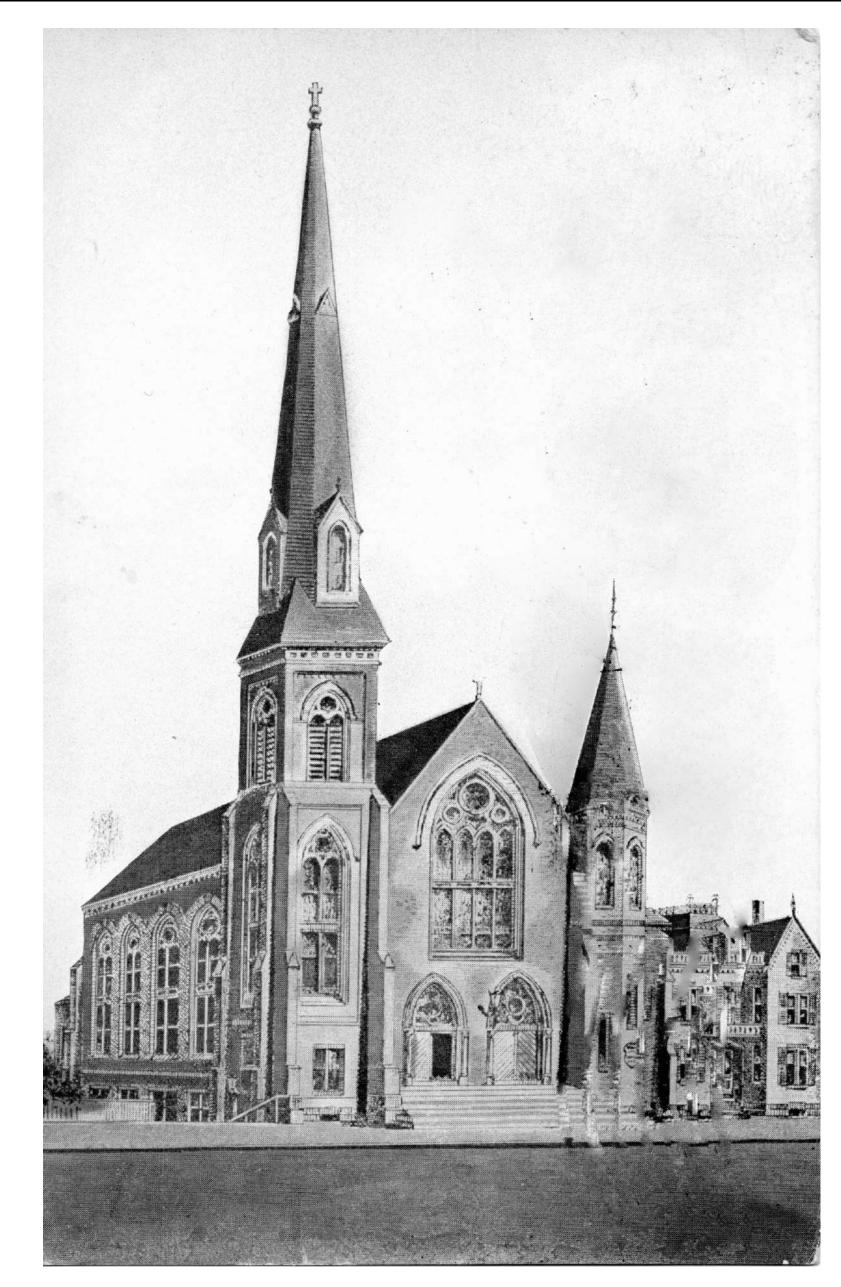
Prepared By:

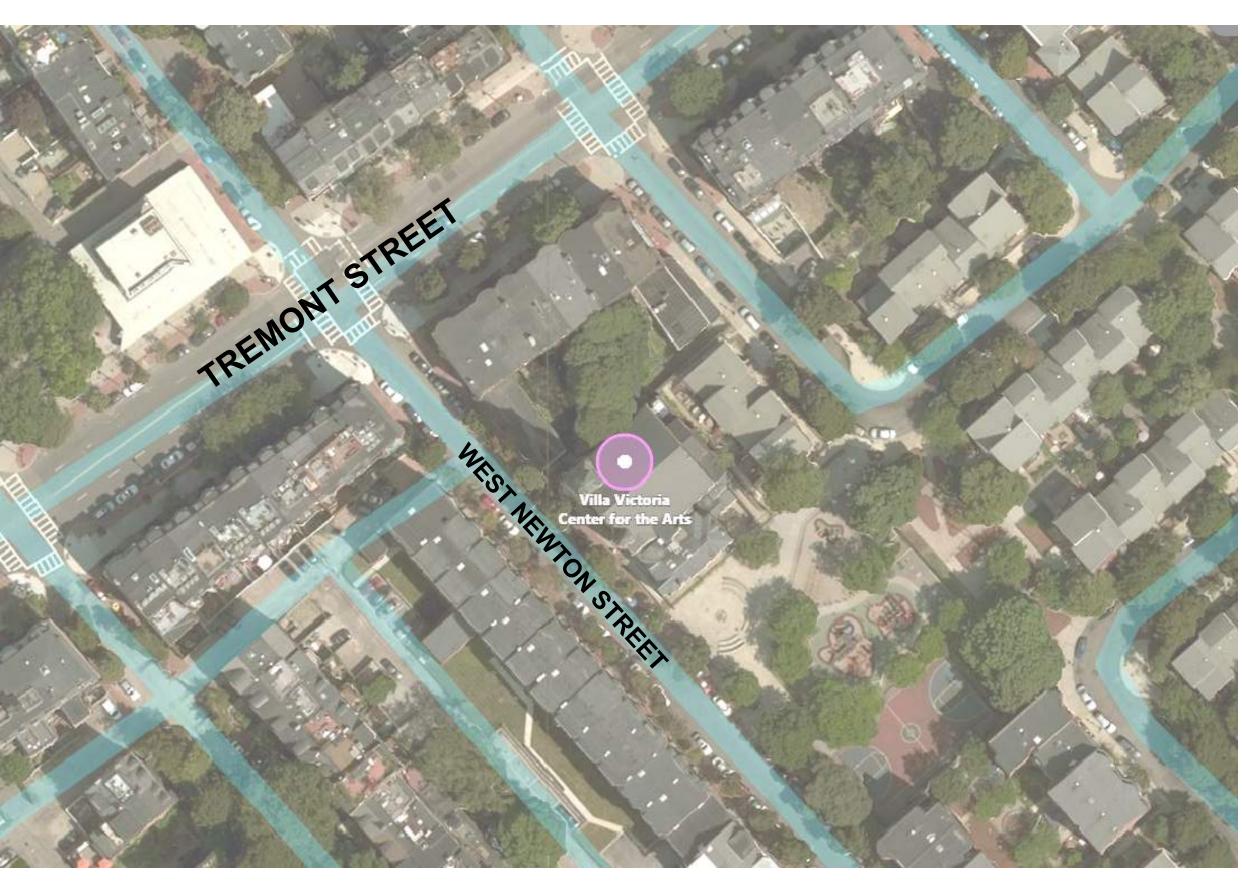


DHK, Inc. - Architecture 54 Canal Street, Boston, MA 02114 617 267 6408 fax. 617 267 1990



Gale Associates, Inc. - Structural Engineer GALE 163 Libbey Parkway, Weymouth, MA 02189 781.335.6465 fax. 781.335.6467





		н	HIGH
ABB	REVIATION	HM	HOLLOW METAL
		HP	HIGH POINT
		HR	HOUR
#	POUND OR NUMBER		HEATING, VENTILATING, AND AIR
&	AND	HVAC	CONDITIONING
@	AT	ILO	IN LIEU OF
Ø	DIAMETER	INSUL	INSULATED OR INSULATION
ACT	ACOUSTIC CEILING FLOOR	INT	INTERIOR
AD	AREA DRAIN	LO	LOW
AFF	ABOVE FINISHED FLOOR	MAX	MAXIMUM
ALUM	ALUMINUM	MO	MASONRY OPEING
ANOD	ANODIZED	MECH	MECHANICAL
BSMT	BASEMENT	MEMBR	MEMBRANE
BYND	BEYOND	MIN	MINIMUM
BOT	BOTTOM	MRGWB	MOISTURE-RESISTANT GYPSUM
CIP	CAST IN PLACE	MKGWD	WALL BOARD
CHNL	CHANNEL	MTL	METAL
CJ	CONTROL JOINT	NIC	NOT IN CONTACT
CLG	CEILING	NO	NUMBER
CLR	CLEAR	NOM	NOMINAL
CMU	CONCRETE MASONRY UNIT	OC	ON CENTER
COL	COLUMN	ОН	OPPOSITE HAND
COMPR	COMPRESSIBLE	OZ	OUNCE
CONC	CONCRETE	PCC	PRE-CAST CONCRETE
CONT	CONTINUOUS	PLUMB	PLUMBING
CPT	CARPET	PLYD	PLYWOOD
СТ	CERAMIC TILE	PT	PRESSURE TREATED
CTYD	COURTYARD	PNT	PAINT OR PAINTED
DBL	DOUBLE	PVC	POLYVINIL CHLORIDE
DEMO	DEMOLISH OR DEMOLISHION	RBR	RUBBER
DIA	DIAMETER	RCP	REFLECTED CEILING PLAN
DIM	DIMENSION	PD	ROOF DRAIN
DIMS	DIMENSIONS	REQD	REQUIRED
DN	DOWN	SAMF	SELF ADHERED MEMBRANE
DR	DOOR		FLASHING
DWG	DRAWING	SIM	SIMILAR
EA	EACH	SPEC	SPECIFIED OR SPECIFICATION
EJ	EXPANSION JOINT	SPK	SPRINKLER
EL	ELEVATION	SST	STAINLESS STEEL
ELEC	ELECTRICAL	STC	SOUND TRANSMISSION COEFFICIENT
ELEV	ELEVATOR	STL	STEEL
	ETHYLENE PROPYLENE DIENE	STRUCT	STRUCTURE
EPDM	M-CLASS	T&G	TONGUE AND GROOVE
EQ	EQUAL	TELE	TELEPHONE
EXIST	EXISTING	TLT	TOILET
EXP JT	EXPANSION JOINT	TO	TOP OF
EXT	EXTERIOR	TOC	TOP OF CONCRETE
FD	FLOOR DRAIN	TOS	TOP OF STEEL
FEC	FIRE EXTINGUISHER CABINET	TPD	TOILET PAPER DISPENSER
FIXT	FIXTURE	T/D	TELEPHONE/DATA
FLR	FLOOR	TYP	TYPICAL
FM	FILLED METAL	UNO	UNLESS NOTED OTHERWISE
FO	FACE OF	U/S	UNDERSIDE
FND	FOUNDATION	VIF	VERIFY IN FIELD
GA	GAUGE	VIF VP	
GALV	GALVANIZED	VP	VISION PANEL

WITH

WOOD

HOLLOW CORE

GYPSUM WALL BOARD

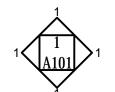
GWB

HC

LEGEND / SYMBOLS



SECTION CUT KEY



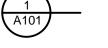
ELEVATION KEY-EXTERIOR



ELEVATION DATUM



DETAIL SECTION/PLAN BUBBLE



ROOM DESIGNATION



DOOR IDENTIFICATION TAG



WINDOW IDENTIFICATION



PHASING PLAN

- 1. Masonry restoration at front façade from the northeast end of the belfry tower and around to the front of the building up to but not including the parish house.
- 2. Replacement of the main spire
- 3. Reconstruction of the main tower masonry belfry as necessary.
- 4. Repairs to the smaller tower as necessary.
- 5. Roofing for the two spires.
- 6. Flashing, roofing etc. necessary at the intersections of the spires with the main
- 7. Front main stairs.
- 8. Railings at main stairs
- 10. Windows on the walls outlined above under item 1.
- 11.Louvers at belfry.
- 12. Main doors and surrounds.

Phase 1B

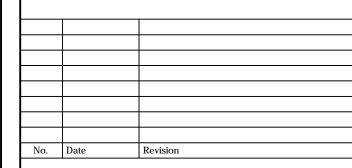
- 1. Masonry restoration at remaining walls around the buildings.
- 2. Remaining roofing, flashings etc.
- 3. Windows along the remaining walls.
- 4. Some selective exterior doors around the periphery of the building. 5. Exterior lighting including architectural lighting.
- 6. Additional exterior power.
- 7. Exterior signage

DRAWING LIST

$C\Omega\Omega\Omega$	Cover Dege
G000	Cover Page
G001	Abbreviations, Legends, Overview of Scope and Drawing List
EX-A-201	Existing Conditions and Demolition Exterior Elevations
EX-A-202	Existing Conditions and Demolition Exterior Elevations
EX-A-203	Existing Conditions and Demolition Exterior Elevations
A-101	NEW WORK AND EXISTING CONDITION STAIR PLANS AND DETAILS
A-200	New Work Exterior Elevations
A-201	New Work Exterior Elevations
A-202	New Work Exterior Elevations
A-500	Enlarged Elevations and Details
A-501	Enlarged Elevations and Details
A-502	Enlarged Elevations and Details
A-600	Phase 1 Window and Door Schedule
EG001	General Information
EG002	Overall Building Elevations
EG003	Overall Building Elevations (cont.)
EA101	Floor Plans
EA201	South Elevation
EA202	West Elevation
EA203	North Elevation
EA204	East Elevation
EA301	Stone and terra Cotta Shape Types
EA511	Small Spire Roof and Cornice Details
EA512	Roof Details
EA513	Typical Synthetic Slate Roof Details
EA521	Typical Building Repair Details
EA531	Masonry Details
EA532	Masonry Details
S-100	Elevation
S-200	Plans
S-201	Plans
S-202	Plans
S-203	Plans
S-300	Sections
S-400	Details
S-401	Details
S-402	Details
S-403	Details
S-404	Details
S600	Steeple Design Criteria
S710	Proposed Demolition & Existing Stair Section
5710 S720	New Stair Plans & Sections
3720 S750	Typical Stair Details
5750 S851	Pilaster Grout Injection Section and Details
S851 S852	Pilaster Strengthening & Rebuilt



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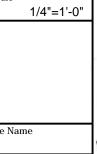


Villa Victoria Center for the Arts

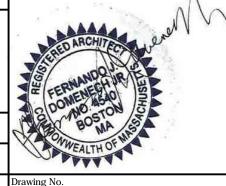
85 WEST NEWTON STREET BOSTON, MA

PHASE 1A CONSTRUCTION DOCUMENTS

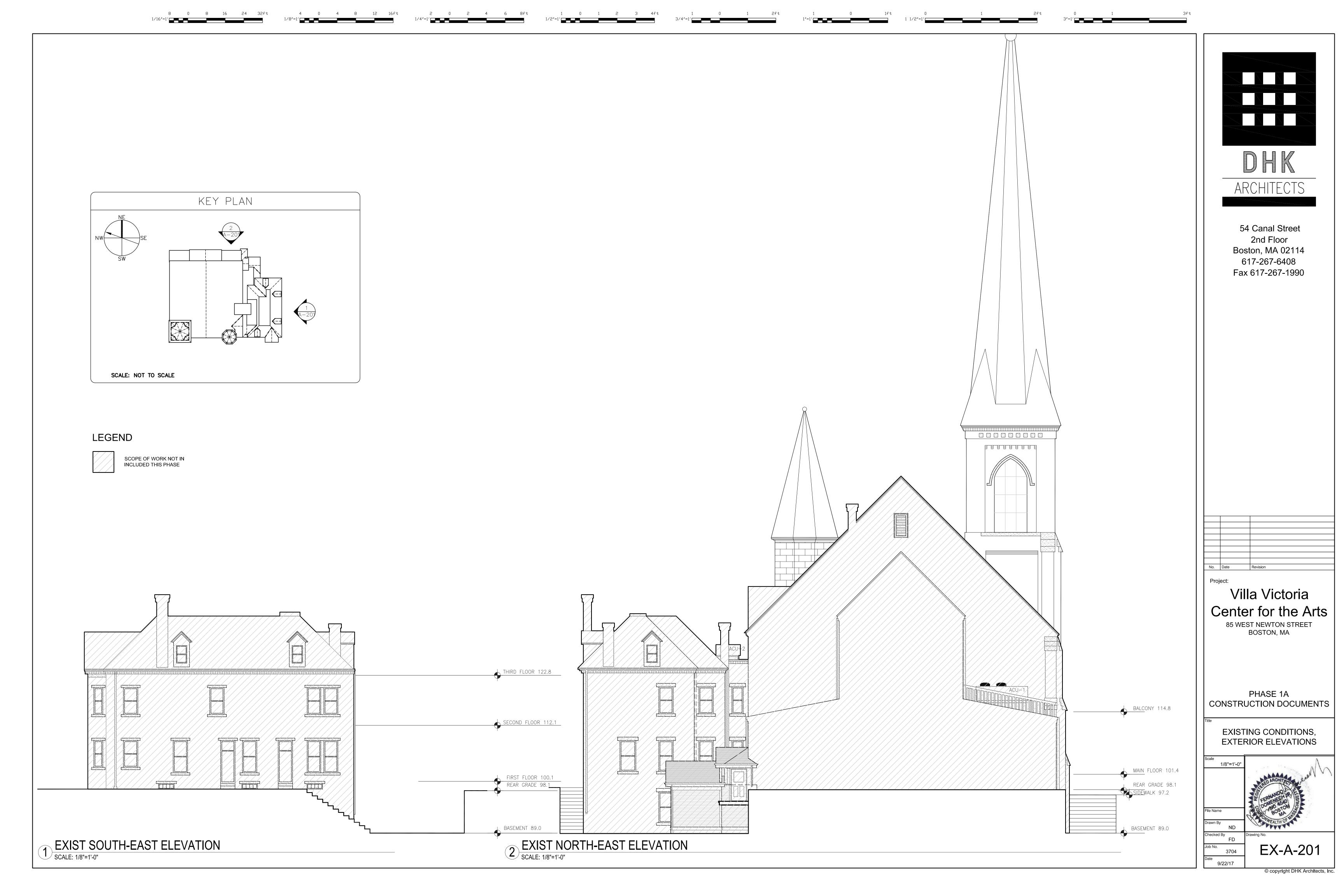
ABBREVIATIONS, LEGENDS, AND DRAWING LIST

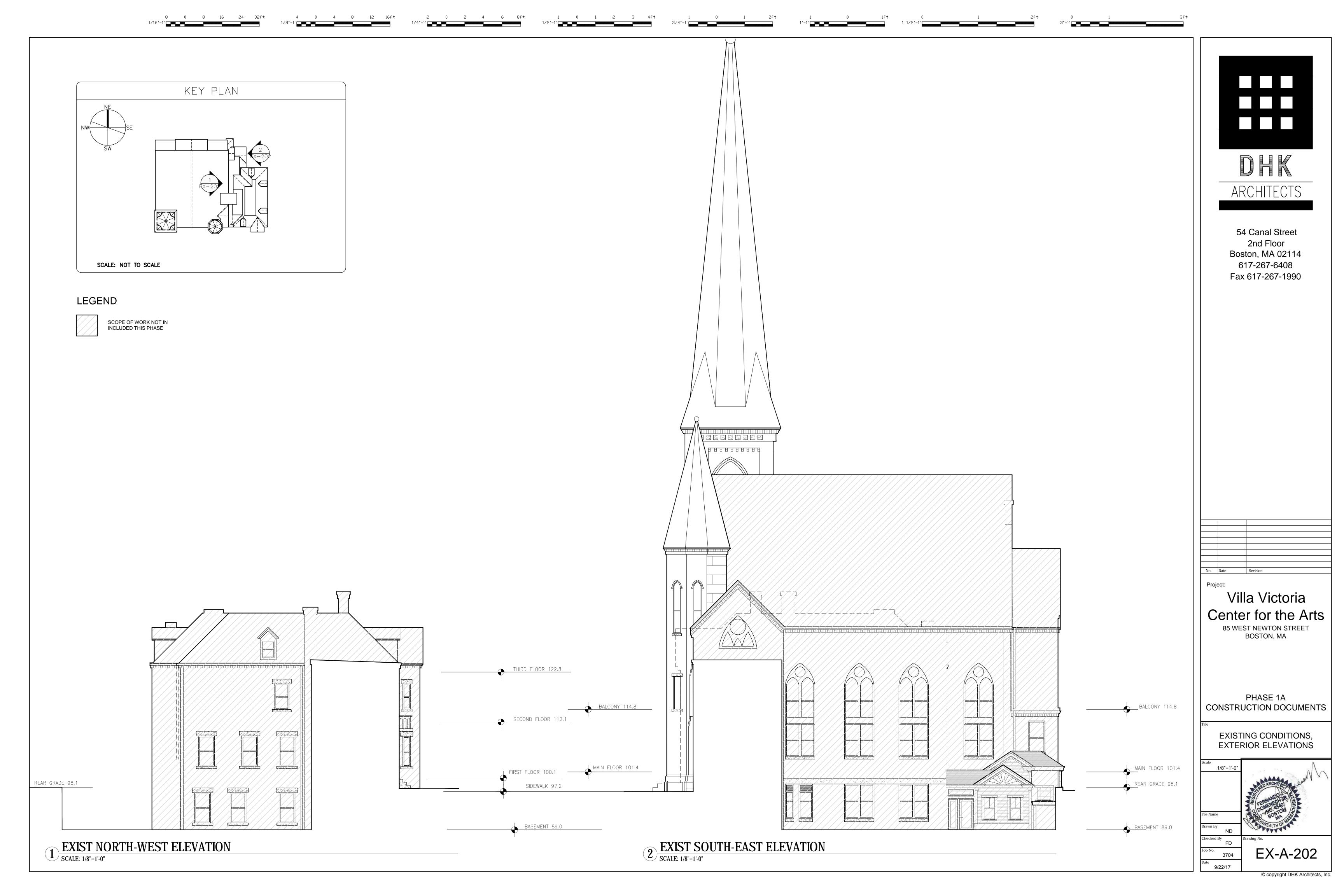


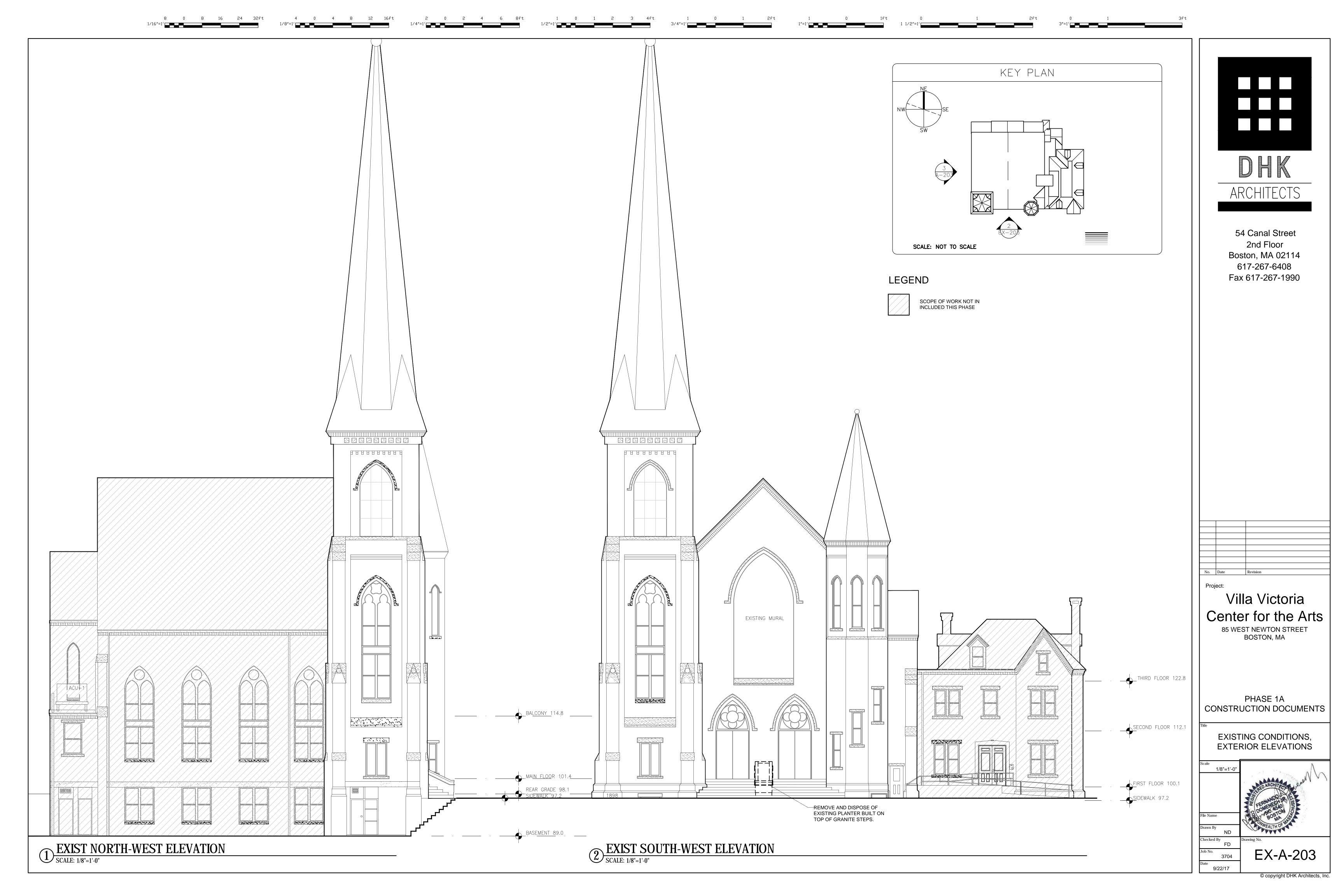
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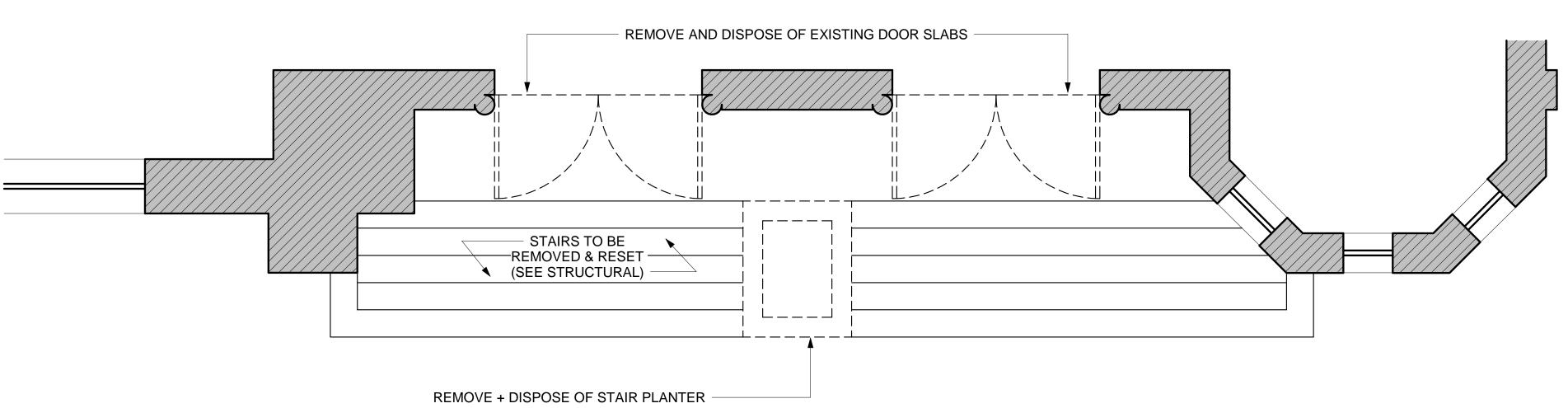


G001 3704









2 0 2 4 6 8ft

1 0 1 2ft 3/4"=1"

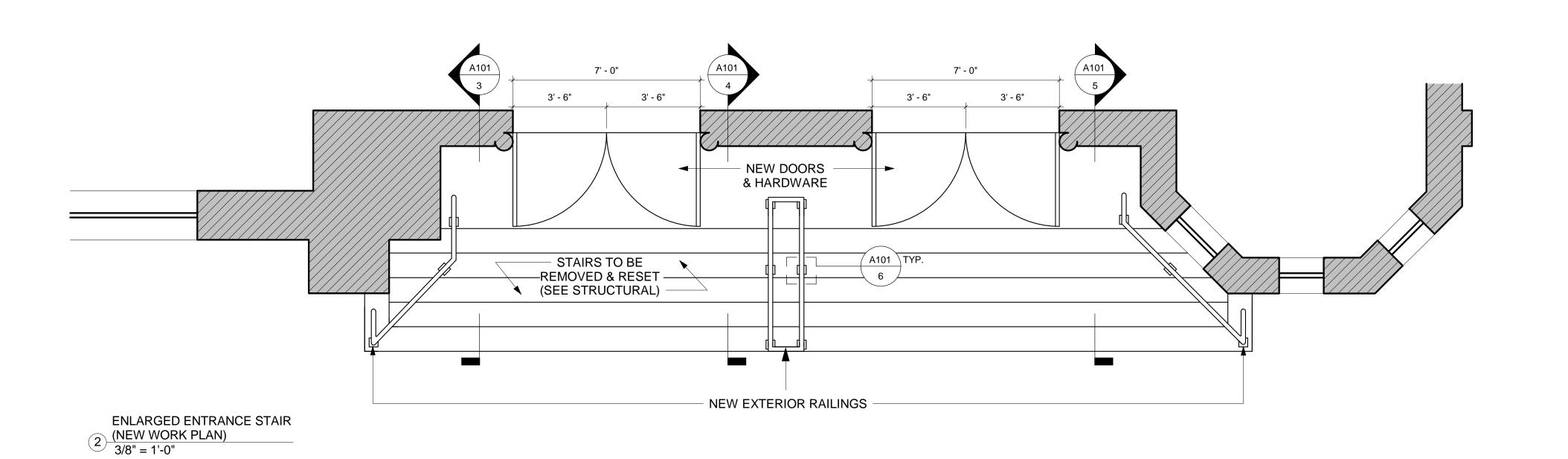
1 0 1 2 3 4ft 1/2"=1"

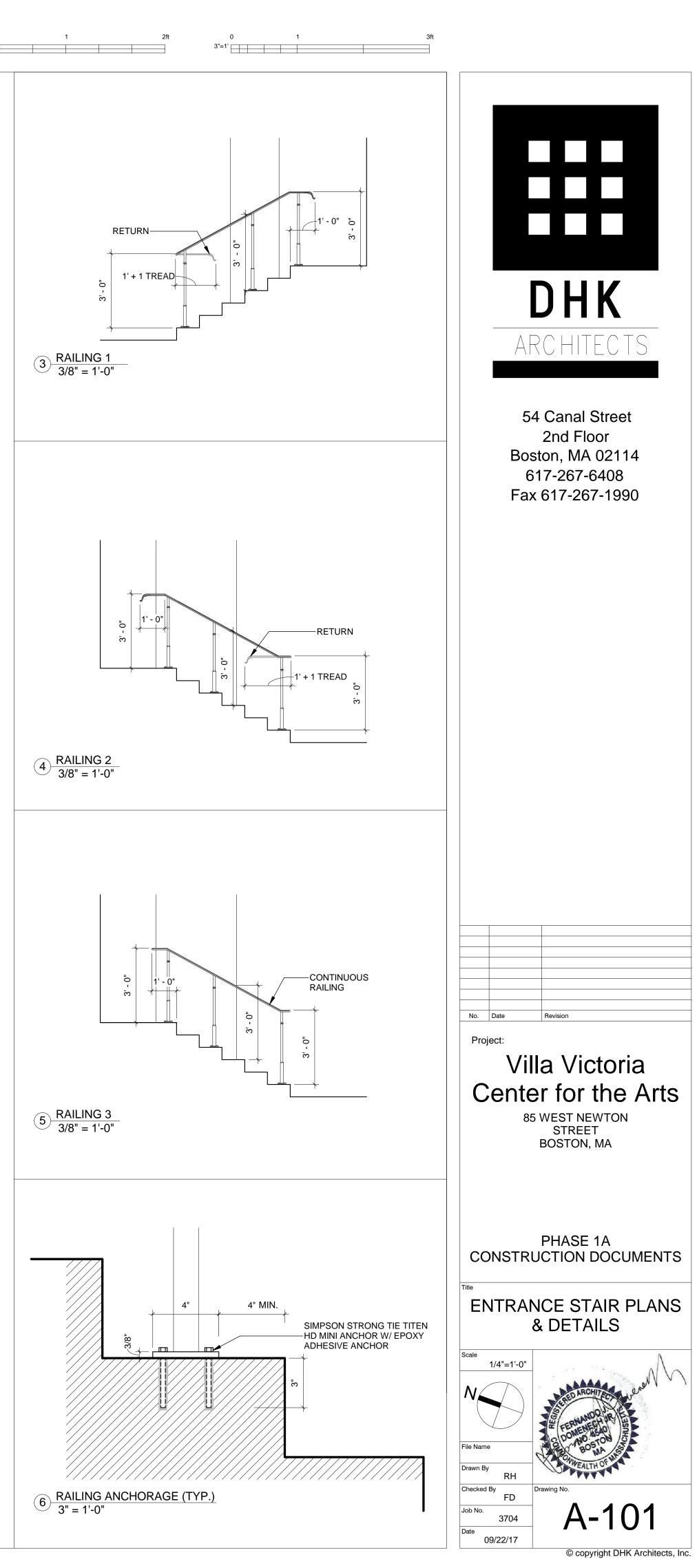
ENLARGED ENTRANCE STAIR

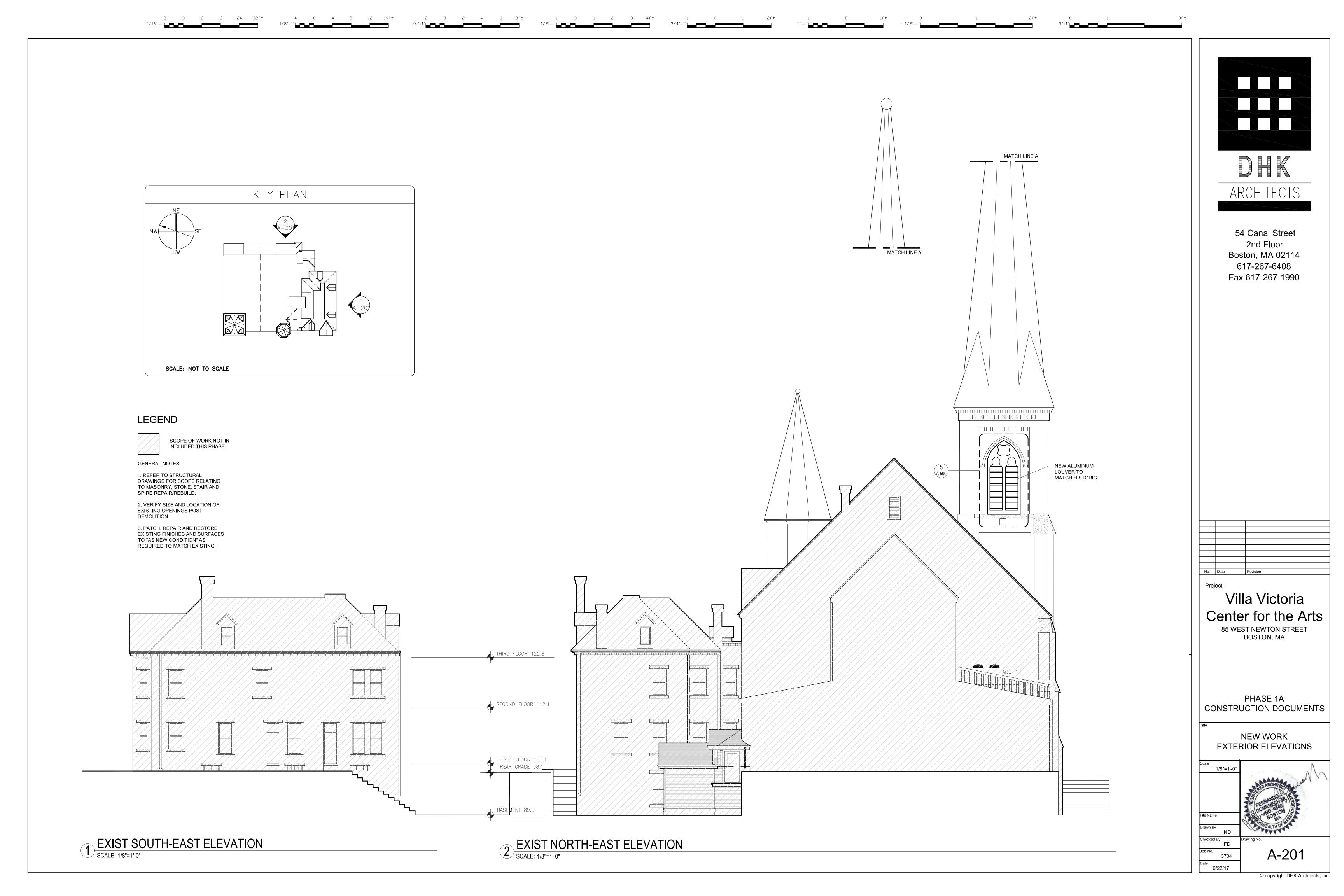
(EXISTING CONDITIONS & DEMO PLAN)

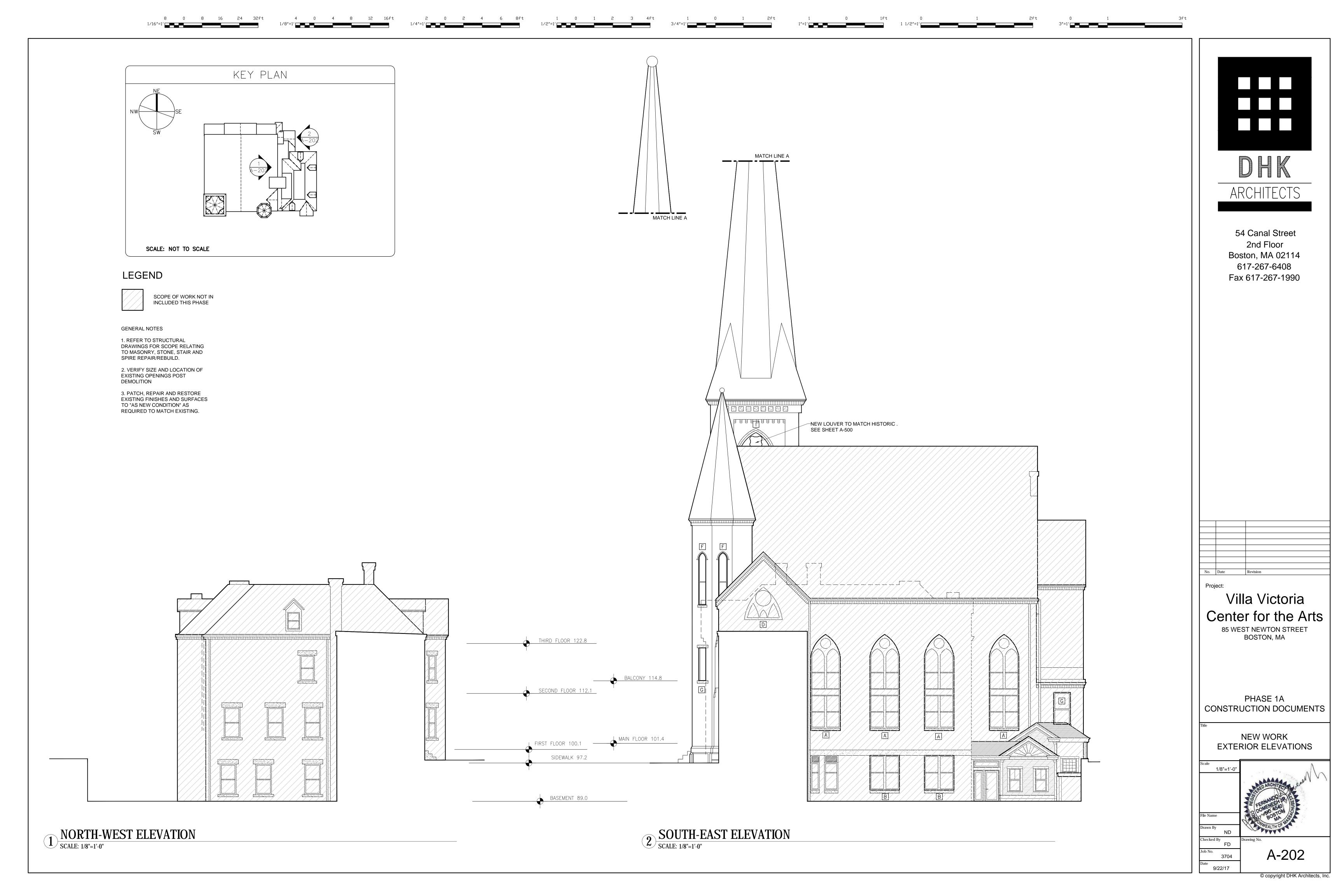
3/8" = 1'-0"

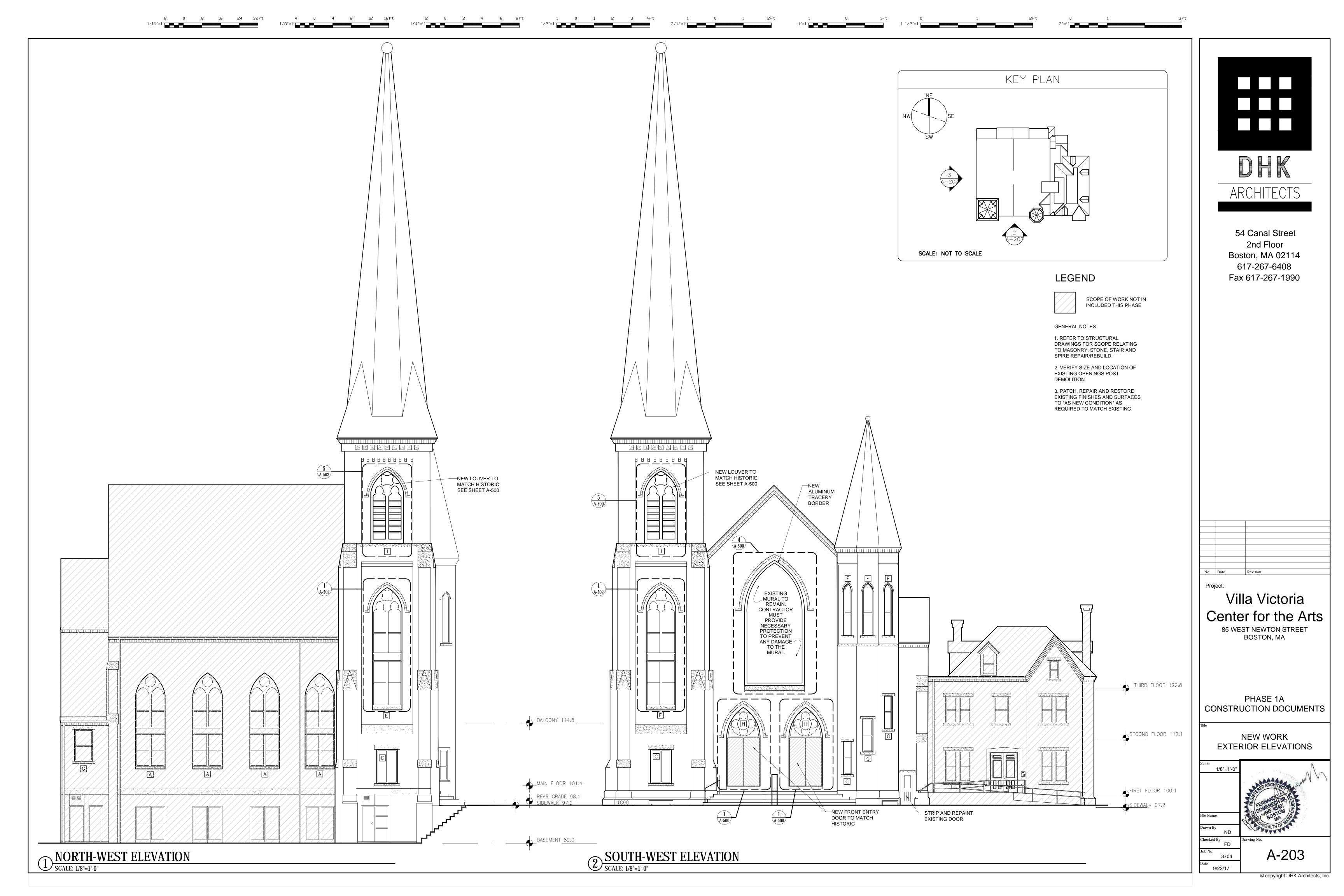
4 0 4 8 12 16ft

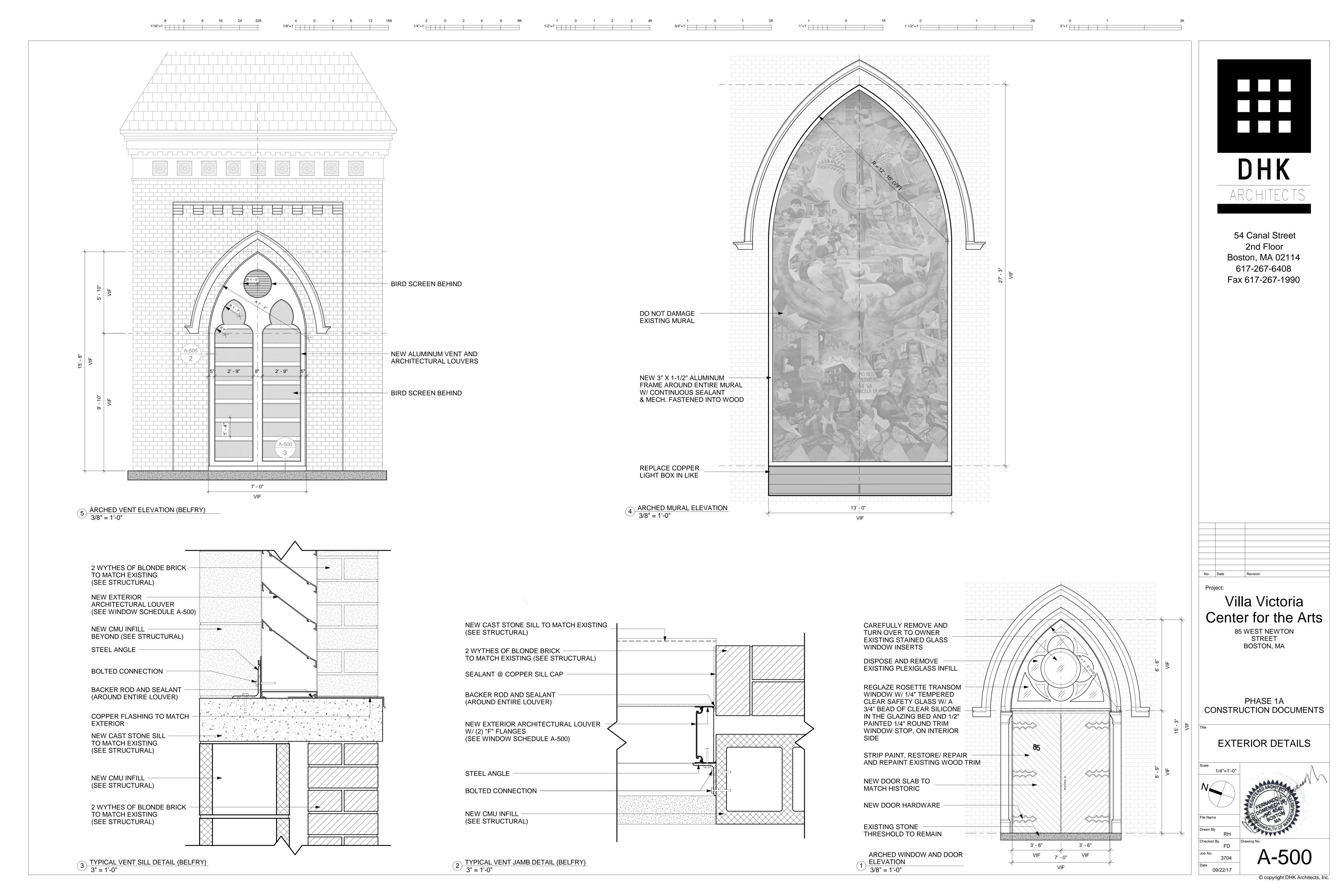


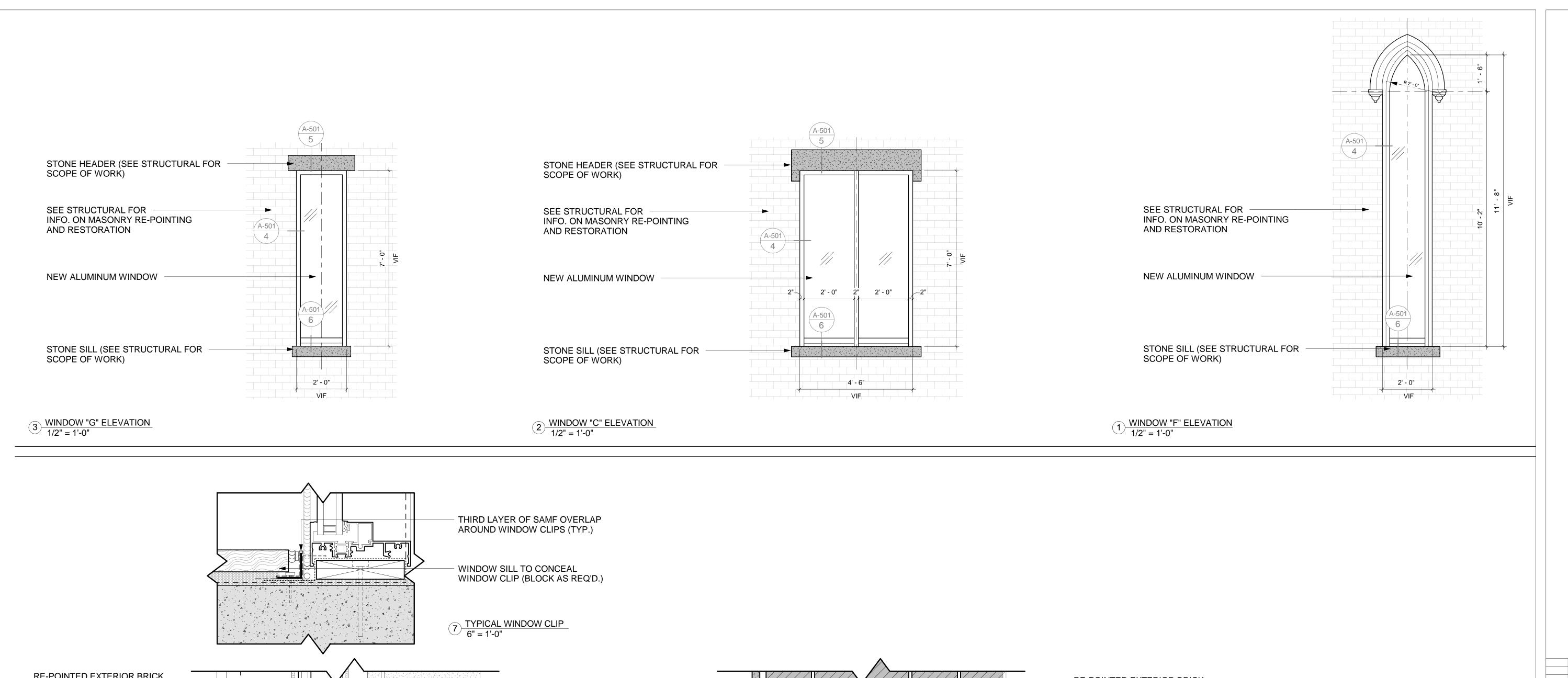








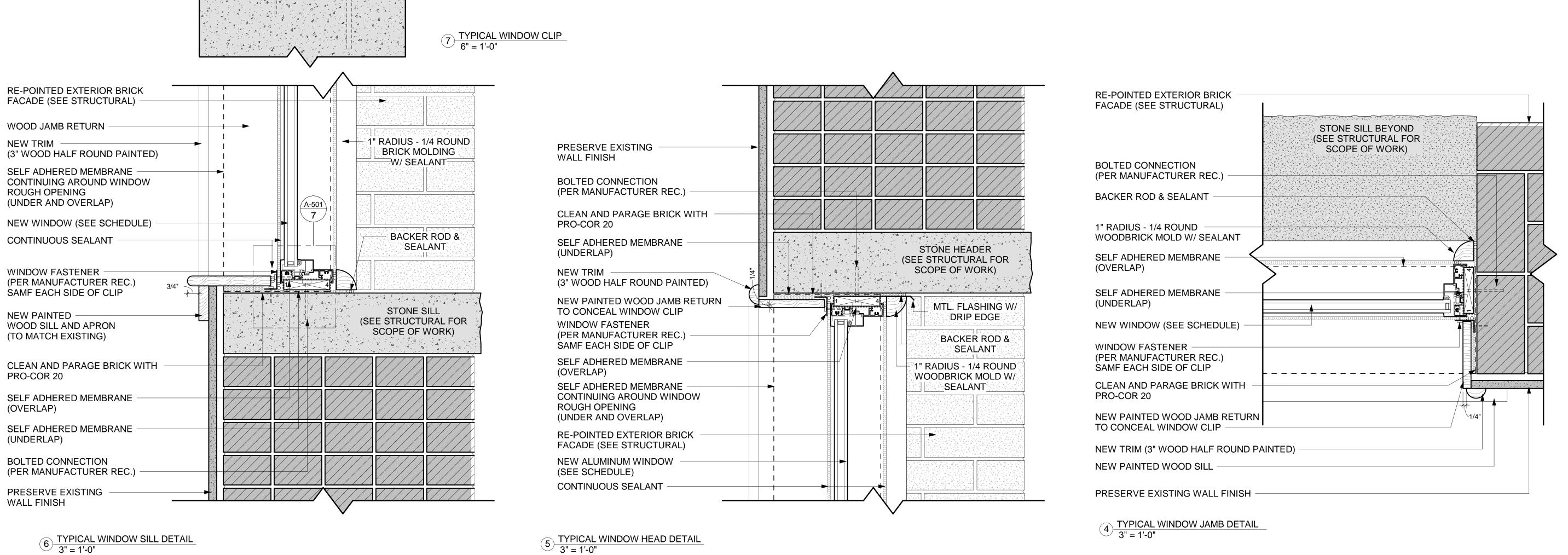




1 0 1 2 3 4ft 1/2"=1"

4 0 4 8 12 16ft

2 0 2 4 6 8ft





3"=1'

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No. Date Revision

Project:

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Center for the Arts

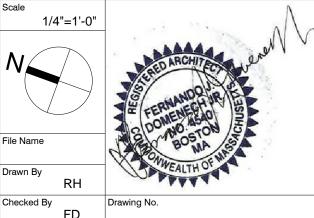
85 WEST NEWTON

STREET

BOSTON, MA

PHASE 1A
CONSTRUCTION DOCUMENTS

TYPICAL WINDOW DETAILS



hecked By FD Drawing No.

3704
ate 09/22/17

RE-POINTED EXTERIOR BRICK FACADE (SEE STRUCTURAL) **BOLTED CONNECTION** STONE SILL BEYOND (SEE STRUCTURAL FOR CONTINUOUS SEALANT W/ SCOPE OF WORK) BACKER ROD SELF ADHERED MEMBRANE CONTINUING AROUND INSIDE PORTIONS OF EXISTING WOOD WINDOW FRAME (UNDER AND OVERLAP) NEW WINDOW (SEE SCHEDULE) CONTINUOUS SEALANT W/ **BACKER ROD** WINDOW FASTENER (PER MANUFACTURER REC.) SAMF EACH SIDE OF CLIP NEW PAINTED WOOD RETURN NEW PAINTED WOOD SILL NEW TRIM (3" WOOD HALF ROUND PAINTED) 1/4" PRESERVE EXISTING WALL FINISH

1/16"=1'

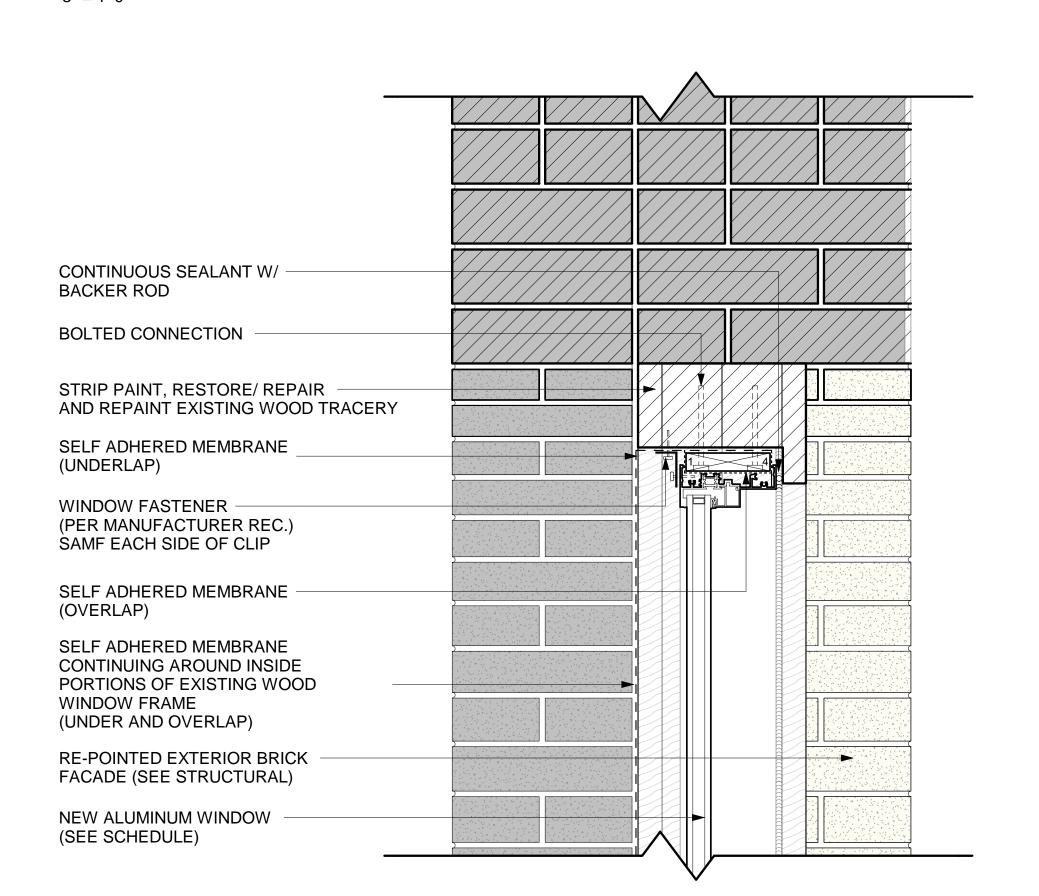
4 0 4 8 12 16ft

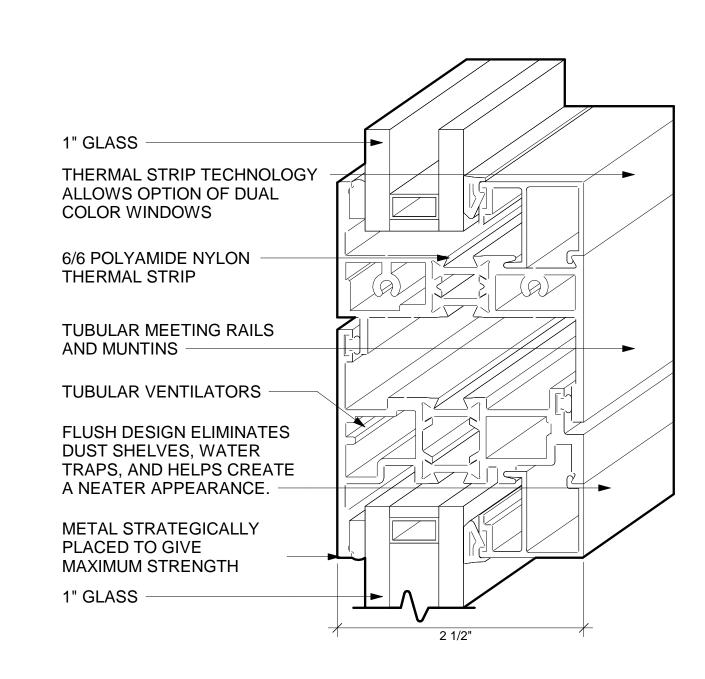
1/8"=1'

2 0 2 4 6 8ft

1/4"=1'

TYPICAL ARCHED WINDOW JAMB $3 \frac{\text{DETAIL}}{3" = 1'-0"}$





3/4"=1'

1 0 1 2 3 4ft

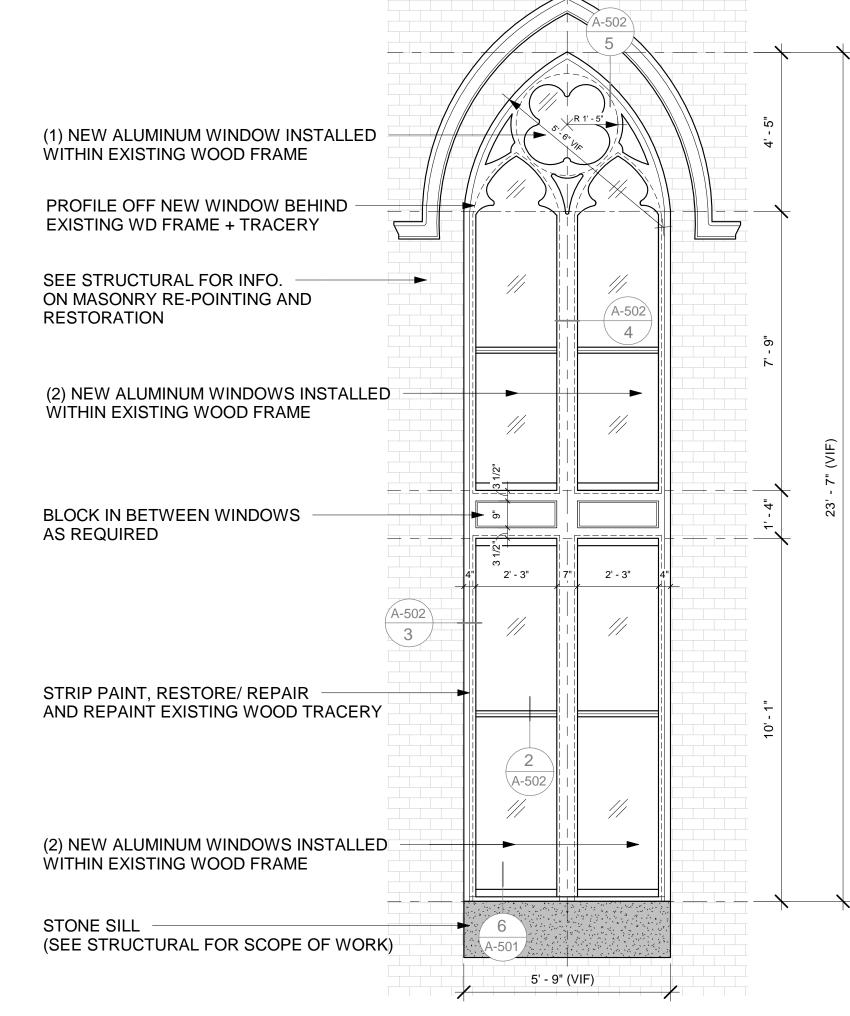
1/2"=1'

- Full 2 1/2" deep frame with double tubular sash, muntins and intermediate rails. (Consult factory for full specifications)

- Thermal Strip technology for added strength and Dual Color finishing capability. - All corners are fastened on each side of the thermal break. Sash corners are mitered, heavy angle reinforced and hydraulically crimped and epoxy welded. - Flush design eliminates dust shelves and water traps and helps create a clean,

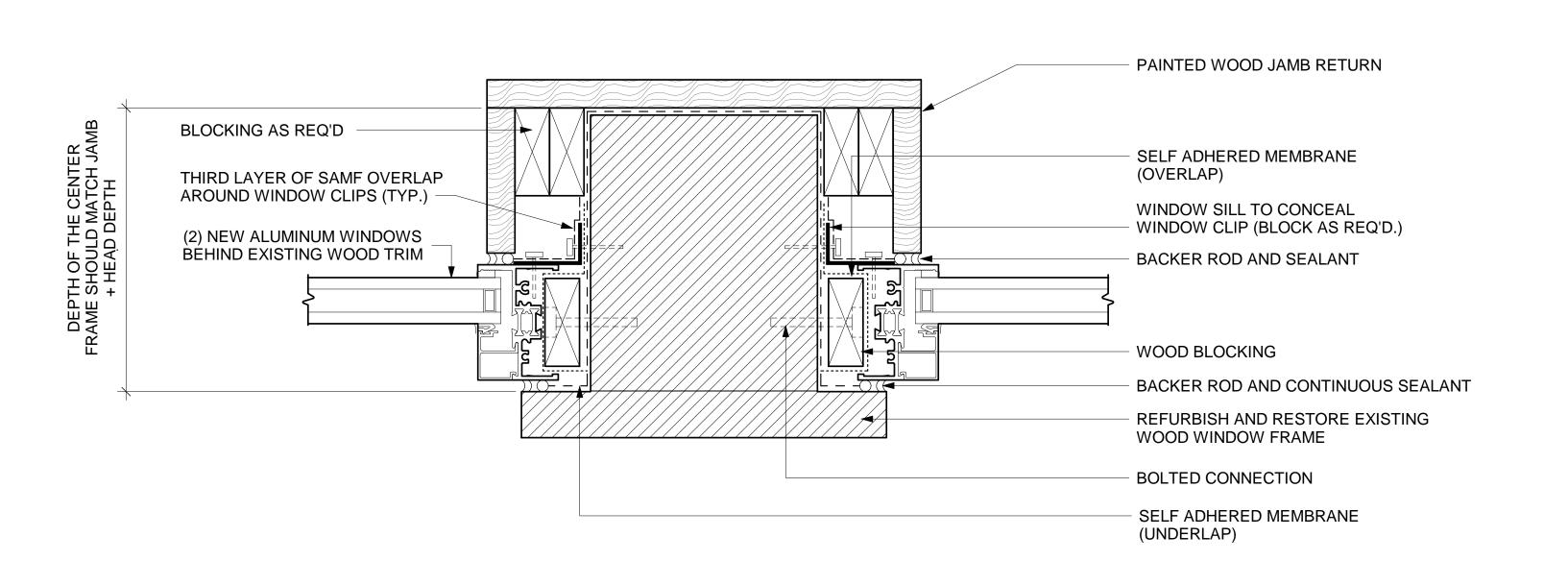
neat appearance. - Each ventilator has two wear resistant nylon glides independant of the hinge to insure maximum alignment and weather tightness.

2 TYPICAL MUNTIN DETAIL DIAGRAM
12" = 1'-0"



3"=1'

1 ARCHED WINDOW ELEVATION 3/8" = 1'-0"



TYPICAL ARCHED WINDOW CENTER

4 JAMB 6" = 1'-0"



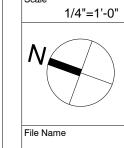
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PHASE 1A CONSTRUCTION DOCUMENTS

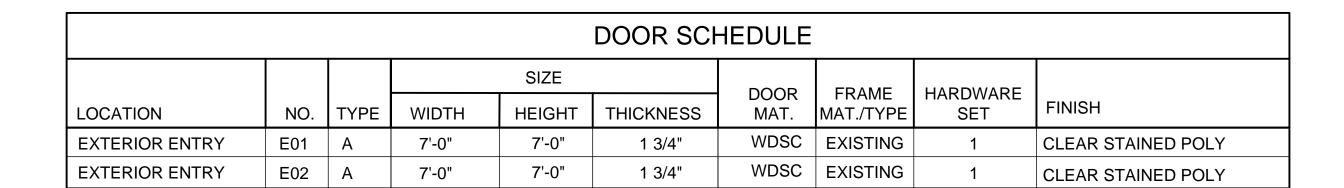
> TYPICAL ARCHED WINDOW DETAILS



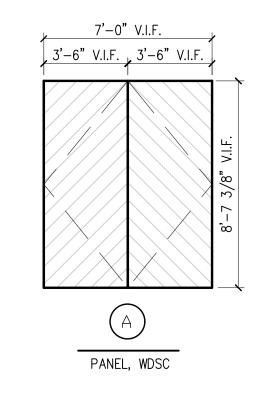
FD 3704 09/22/17

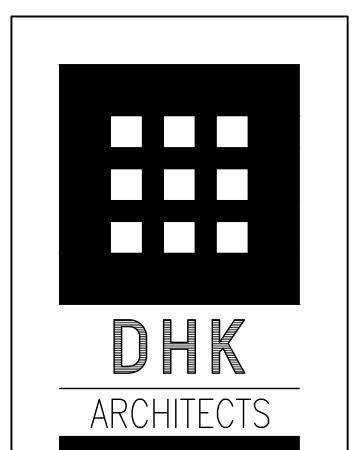
5 DETAIL
3" = 1'-0"

TYPICAL ARCHED WINDOW HEAD



			HARDWARE SCHE	DULE	
HARDWARE	HARDWARE SET	MANUFACTURER	MODEL NUMBER	FINISH	REMARKS
CRASH BAR/ LOCK/PULL	1	YALE	1 each: 1530-L8F x AU673F x RHR x 722, plus 1 each 1510F x AU635F x LHR x 722	BLACK OXIDIZED BRONZE, OIL RUBBED	
CLOSER	1	SHIELD SECURITY	#801202	BRONZE	
HINGE	1	BALDWIN	#1045.402	OIL RUBBED BRONZE	
HINGE STRAP	1	ROCKY MOUNTAIN	#OH5112BD	OIL RUBBED BRONZE	1 STRAP PER HINGE





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PHASE 1A WINDOW SCHEDULE PHASE 1B WINDOW SCHEDULE Villa Victoria Center for the Arts 85 WEST NEWTON STREET BOSTON, MA PHASE 1A 6'-4" V.I.F. CONSTRUCTION DOCUMENTS PHASE 1 WINDOW AND DOOR SCHEDULES 1/8"=1'-0" 6′-4″ V.I.F. 7′−1″ ∨.I.F. 6'-4" V.I.F. 7′-0″ ∨.I.F. 8'-2 1/8" V.I.F. WINDOW TYPE D ALUMINUM FIXED WINDOW TYPE G ALUMINUM FIXED WINDOW TYPE A ALUMINUM DOUBLE WINDOW TYPE B WINDOW TYPE H WOOD FIXED WINDOW TYPE C ALUMINUM FIXED METAL LOUVER TYPE i COUNT: 4 WINDOW TYPE F ALUMINUM FIXED WINDOW TYPE E ALUMINUM DOUBLE ALUMINUM FIXED WINDOW REPLACEMENT COUNT: 4 WINDOW **HUNG WINDOW** DOUBLE-HUNG WINDOW REPLACEMENT WINDOW GLAZING WINDOW WINDOW REPLACEMENT WITH FIXED GLASS WINDOW REPLACEMENT COUNT: 2 REPLACEMENT COUNT: 4 REPLACEMENT WITH STAINED GLASS INSERTS COUNT: 1 REPLACEMENT COUNT: 2 ROSETTE AT TOP REPLACEMENT WITH STAINED COUNT: 2 A-600 3704 **GLASS INSERTS** COUNT: 8

GENERAL NOTES

- 1. THE INFORMATION SHOWN ON THIS DRAWING HAS BEEN COMPILED FROM VARIOUS SOURCES, AND MAY NOT REFLECT THE ACTUAL CONDITIONS AT THE TIME OF CONSTRUCTION.
- 2. FOR THE SAKE OF CLARITY, EACH INDIVIDUAL DETAIL HAS NOT BEEN INDICATED. INSTALLATION DETAILS HAVE BEEN INDICATED FOR TYPICAL COMPONENTS.
- 3. HATCH PATTERNS ARE FOR REPRESENTATION ONLY.
- 4. ALL ITEMS OF CONSTRUCTION SHALL BE IN ACCORDANCE WITH LOCAL, STATE AND FEDERAL REGULATIONS APPLICABLE TO THE PROJECT.
- 5. THE CONTRACTOR SHALL REPORT DETERIORATED OR UNSUITABLE SUBSTRATES TO THE OWNER PRIOR TO PERFORMING WORK.
- 6. THE CONTRACTOR IS CAUTIONED THAT DUE TO BUILDING OCCUPANCY, THE OWNER REQUIRES COMPLIANCE ON WORK HOURS, SCHEDULING, SET—UP, CLEANUP, PARKING, SECURITY, ETC. REFER TO SPECIFICATIONS FOR OWNER REQUIREMENTS.
- 7. THE WORK SHALL BE 100% WEATHER TIGHT ON A DAILY BASIS. REMOVE ONLY AS MUCH WORK AS CAN BE REPLACED ON THE SAME DAY. PHASED CONSTRUCTION IS NOT PERMITTED.
- 8. FOR THE SAKE OF CLARITY, SECUREMENT FASTENERS ARE NOT SHOWN ON THE DETAIL DRAWINGS. REFER TO THE SPECIFICATIONS FOR FASTENER TYPES AND SPACING.
- 9. REMOVE EXISTING DESIGNATED SLATE SHINGLES, LOW SLOPED ROOF SYSTEMS, SHEET METAL EDGE COMPONENTS AND ASSOCIATED FLASHINGS AND UNDERLAYMENTS DOWN TO THE EXISTING WOOD ROOF DECK TO INSTALL THE NEW ROOF SYSTEM, AND NEW LOW SLOPE EPDM ROOF SYSTEMS.
- 10. DETAILS NOT DEPICTED SHALL BE CONSTRUCTED IN A MANNER CONSISTENT WITH THE DETAIL DRAWINGS.
- 11. REFER TO CONTRACT DOCUMENTS FOR SCHEDULE LIMITATIONS OF EXTERIOR WORK.
- 12. THE CONTRACTOR IS RESPONSIBLE FOR CHECKING AND VERIFYING ALL DIMENSIONS, CONDITIONS, HEIGHTS, MATERIAL THICKNESSES, ETC., IN THE FIELD PRIOR TO SUBMITTING THEIR BID AND COMMENCING CONSTRUCTION.
- 13. THE BUILDING SHALL BE MAINTAINED IN A DUST FREE CONDITION. THE CONTRACTOR SHALL PROVIDE ALL NECESSARY TEMPORARY PROTECTION TO ENSURE THE INTERIOR OF THE BUILDING REMAINS DUST FREE. NO DEMOLITION CAN PROCEED UNTIL AN APPROVED TEMPORARY PROTECTION SYSTEM IS IN PLACE.

MASONRY REPAIR NOTES

- 1. THIS PROJECT INVOLVES HISTORIC MASONRY ASSEMBLIES. REFER TO THE SPECIFICATIONS FOR REQUIREMENTS TO MATCH THE EXISTING HISTORIC MORTAR, AS WELL AS REPLACEMENT STONE AND BRICK.
- 2. THE CONTRACTOR MUST SUBMIT ENGINEERS SEALED (CERTIFIED) SHORING SHOP DRAWINGS FOR MASONRY REMOVALS/ REBUILDING PRIOR TO THE PERFORMANCE OF ANY WORK
- 3. METHODS OF DEMOLITION MUST NOT APPLY EXCESSIVE IMPACT, FORCE OR VIBRATION TO EXISTING MASONRY WALLS AND RELATED BUILDING CONSTRUCTION. THE CONTRACTOR MUST SUBMIT PROPOSED APPARATUS, EQUIPMENT, MEANS AND METHODS TO THE OWNER AND GALE FOR REVIEW AND APPROVAL PRIOR TO INITIATING THE DEMOLITION WORLD
- 4. IN NO CASE SHALL SAWCUTTING, HAMMER CHIPPING, DRILLING AND OTHER METHODS OF DEMOLITION DAMAGE SURROUNDING MASONRY OR BUILDING COMPONENTS WHICH ARE TO REMAIN. ANY AND ALL EXISTING ROOFING, MATERIALS OR BUILDING COMPONENTS WHICH ARE DAMAGED OR ADVERSELY AFFECTED BY OVERCUTS, CRACKS, OR OTHER DETRIMENTAL CONDITIONS AS A RESULT OF THE WORK MUST BE REPAIRED OR REPLACED BY THE CONTRACTOR AT NO ADDITIONAL COST.
- 5. NEW MASONRY CONSTRUCTION MUST BE PLACED TO REPLICATE EXISTING COURSING GEOMETRY AS A MINIMUM; UNLESS OTHERWISE INDICATED IN THESE DESIGN DOCUMENTS. COURSES MUST BE SOLID AND UNDAMAGED, AND SOLIDLY MORTARED ALL AROUND. DAMAGED, CRACKED, SPALLED OR OTHERWISE DEFECTIVE COURSES MUST BE REPLACED WITH NEW SPECIFIED UNITS, SOLIDLY MORTARED IN PLACE.
- 6. MASONRY UNITS WHICH REQUIRE GEOMETRICAL MODIFICATION AROUND EXISTING OR NEW EMBEDMENTS MUST BE SHAPED TO ACHIEVE A GEOMETRICALLY PLUMB AND TRUE CONDITION. HAMMER BREAKING OF MASONRY UNITS RESULTING IN JAGGED EDGES OR UNEVEN SURFACES IS NOT ALLOWED.
- 7. DURING MASONRY REPAIRS, IF ADDITIONAL DEFICIENT, SOFT, DETERIORATED, DAMAGED, OR SEGREGATED MASONRY CONDITIONS ARE ENCOUNTERED, NOTIFY THE OWNER AND ENGINEER FOR REVIEW PRIOR TO APPLYING NEW REPAIRS.

ELEVATION NOTES

- 1. THE ELEVATIONS ARE PROVIDED TO SHOW APPROXIMATE QUANTITIES AND CONFIGURATIONS. THE ACTUAL SIZES AND CONFIGURATION WILL VARY FROM THOSE SHOWN. THE CONTRACTOR SHALL BASE THE BID AND CONTRACT AMOUNTS ON ACTUAL FIELD CONDITIONS.
- 2. GRAPHIC HATCHING AND SYMBOLS ARE USED ON THE ELEVATION DRAWINGS TO INDICATE APPROXIMATE AREAS INCLUDED IN THE SCOPE OF WORK. ACTUAL DIMENSIONS AND QUANTITIES SHALL BE FIELD VERIFIED. REFER TO THE LEGEND ON THIS SHEET FOR THE CORRESPONDING SCOPE OF WORK ASSOCIATED WITH A PARTICULAR HATCH OR SYMBOL.
- 3. CONTRACTOR IS RESPONSIBLE FOR DESTROYING AND REMOVING BEES/HORNETS/WASPS WHICH MAY BE PRESENT.
- 4. REMOVE AND REPLACE SPECIFIC STONE MASONRY UNITS AS DESIGNATED ON THE ELEVATIONS.
- 5. REMOVE ABANDONED STEEL ANCHORS AND OTHER EMBEDMENTS AS DESIGNATED. REMOVE AND REPLACE DETERIORATED STONE MASONRY UNITS WHERE DAMAGED FROM EMBEDMENTS (PATCHING IS NOT ALLOWED).
- 6. REMOVE VEGETATION FROM BUILDING AS PART OF THE COMPREHENSIVE MASONRY RESTORATION CLEANING.

CODE INFORMATION

THIS PROJECT ENTAILS REPAIRS TO THE EXTERIOR ENVELOPE OF THE BUILDING. CONSTRUCTED APPROXIMATELY C.1898 (SEE CORNERSTONE), THE FACILITY IS WITHIN BOSTON'S SOUTH END LANDMARKS DISTRICT, AND AS SUCH, INHERITS THE RESTRICTIONS AND GUIDELINES AS PROVIDED BY THE DISTRICT. INTENT SHALL BE FOR ALL WORK TO MEET THE REQUIREMENTS OF THE SECRETARY OF INTERIORS STANDARDS FOR HISTORIC PRESERVATION: REHABILITATION FOR NEW WORK, AND RESTORATION FOR REPLACEMENT AND REPAIR WORK.

THIS PROJECT CONFORMS TO THE 9TH EDITION MSBC FOR REPAIRS, 2014 IEBC AS AMENDED FOR REPAIRS AND ALTERATIONS. REFER TO STRUCTURAL DRAWINGS FOR SPECIFIC CODE REQUIREMENTS OF THOSE PORTIONS OF THE WORK.

PHASE 1A WORK GENERALLY INCLUDES:

- REPLACEMENT OF THE TOWER SPIRE.
 RESTORATION AND REPAIRS TO MASONRY ASSEMBLIES OF THE 1895 BUILDING
- WALLS AND STEPS.

 ROOF REPLACEMENT AT THE TWO TOWERS
- REPLACEMENT OF PILASTERS ON WEST NEWTON ST. AND ALL SIDES OF THE TOWER.
 REPLACEMENT OF THE TOP PORTION OF THE LARGE TOWER MASONRY ASSEMBLIES WITH NEW CONSTRUCTION.
- MASONRY WALL REPAIRS AND RESTORATION

NEW ROOFING SYSTEMS NOTE: ALL ITEMS ARE NEW UNLESS DESIGNATED AS EXISTING ROOFS ARE CONSIDERED TEMPORARY FOR PROTECTION BETWEEN PHASED CONSTRUCTION AND ARE TO REMAIN IN PLACE AFIER CONSTRUCTION. SCALE: 3"=1'-0" EPPOM ROOF MEMBRANE 1" RIGID INSULATION VAPOR RETARDER (ADHERED) CONCRETE COMPOSITE DECK BELFRY FLOOR PMMA ROOF MEMBRANE CONCRETE COMPOSITE DECK



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Gale Associates, Inc .
Engineers and Planners

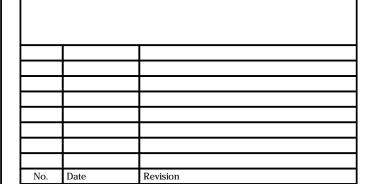
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Boston Baltimore Orlando Hartford

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Project

Villa Victoria
Center for the Arts

85 WEST NEWTON STREET

BOSTON, MA

GENERAL INFORMATION

PHASE 1A

CONSTRUCTION DOCUMENTS

AS NOTED

Vile Name

Drawn By CAC MDF
KPB EWM SMF
Checked By
MDF CM
Job No. 3704
GALE Job No. 832681

9/22/2017

EG001

COREY G.



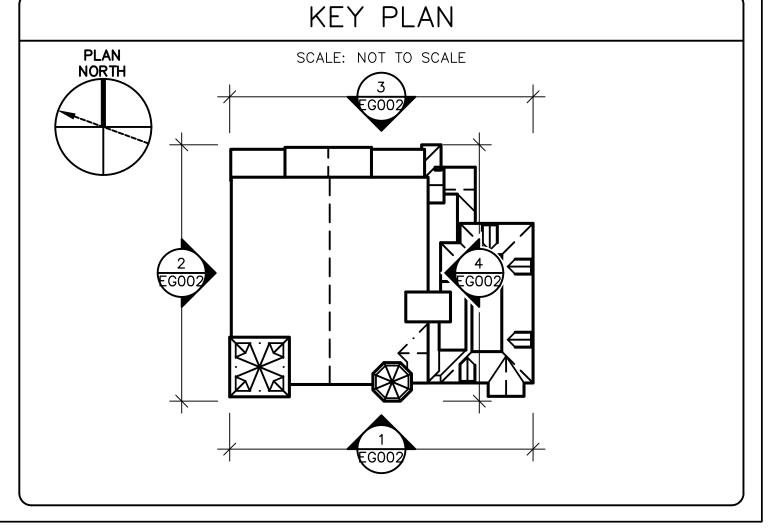


GENERAL NOTES

- 1. PHASE 1A WORK IS LIMITED TO THE MAIN TOWER AND ASSOCIATED SPIRE, SMALL TOWER AND ASSOCIATED SPIRE, AND THE WEST NEWTON STREET OLD CHURCH SOUTH ELEVATION. ROOFING AND REMAINING ELEVATION WORK, INCLUDING THE PARISH HOUSE, WILL BE A PART OF PHASE 1B SCOPE OF WORK.

 2. THE INTENT OF THIS PAGE IS TO PROVIDE PHOTOGRAPHIC REPRESENTATION OF THE BUILDING ELEMENTS BEING REMOVED AT THE TOWER. COMPONENTS REQUIRE MATCHING ORIGINAL CONSTRUCTION AS PAT OF THE RECONSTRUCTION WORK. THE CONTRACTOR SHALL COORDINATE THE REBUILDING OF OTHER AREAS WITH CONTRACTOR CONSTRUCTION PHOTOGRAPHS TO BE TAKEN PRIOR TO DEMOLITION
- PHOTOGRAPHS TO BE TAKEN PRIOR TO DEMOLITION.

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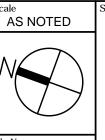
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No. Date Revision

85 WEST NEWTON STREET BOSTON, MA

PHASE 1A CONSTRUCTION DOCUMENTS

REF. BUILDING ELEVATIONS



Drawn By CAC MDF KPB EWM SMF Checked By MDF CM

Job No. 3704 EG002 GALE Job No. 832681 9/22/2017



SEE DRAWING 1/EA203 FOR ENLARGED ELEVATION



SEE DRAWING 1/EA204 FOR ENLARGED ELEVATION

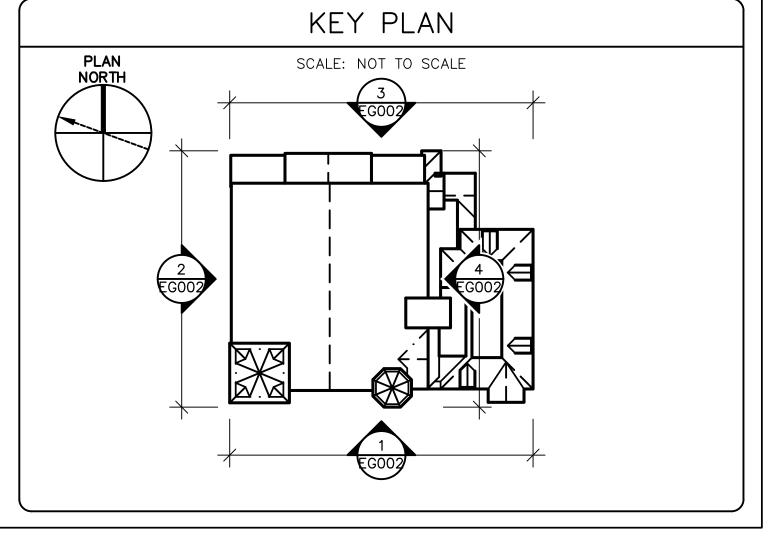


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- 1. PHASE 1A WORK IS LIMITED TO THE MAIN TOWER AND ASSOCIATED SPIRE, SMALL TOWER AND ASSOCIATED SPIRE, AND THE WEST NEWTON STREET OLD CHURCH SOUTH ELEVATION. ROOFING AND REMAINING ELEVATION WORK, INCLUDING THE PARISH HOUSE, WILL BE A PART OF PHASE 1B SCOPE OF WORK.

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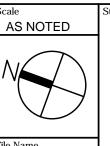
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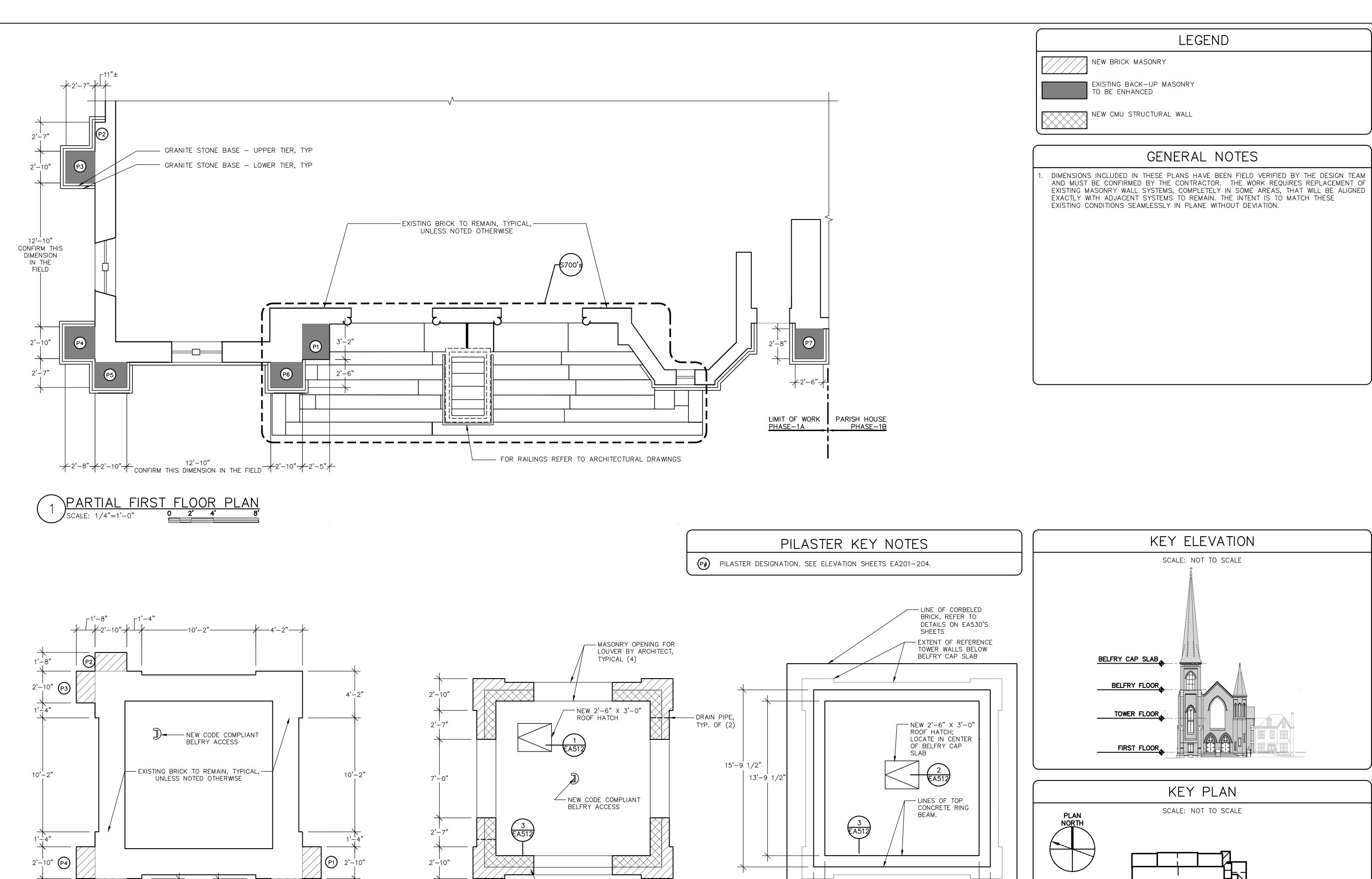
REF. BUILDING ELEVATIONS



Drawn By CAC MDF KPB EWM SMF Checked By
MDF CM Job No. 3704 GALE Job No. 832681

9/22/2017

EG003



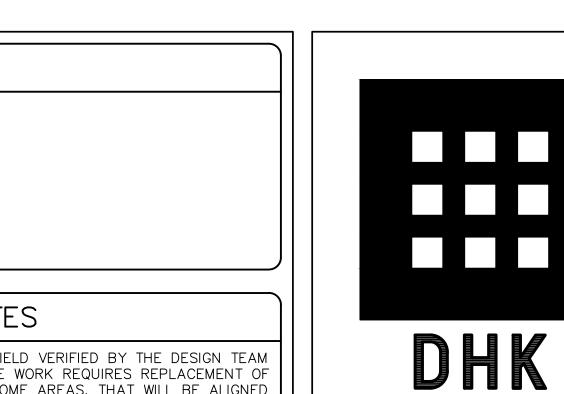
- REFERENCE WALL ASSEMBLY: 12" NOM. CMU BACK-UP, 1/2" AIRSPACE/MORTAR NET,

4" BACK-UP BRICK TIED TO CMU

4" FACE BRICK HEADERED TO BACK-UP BRICK.

—13[']-9-1/2"-

_15'-9-1/2"₋



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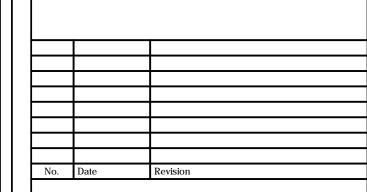


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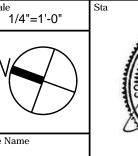


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PHASE 1A **CONSTRUCTION DOCUMENTS**

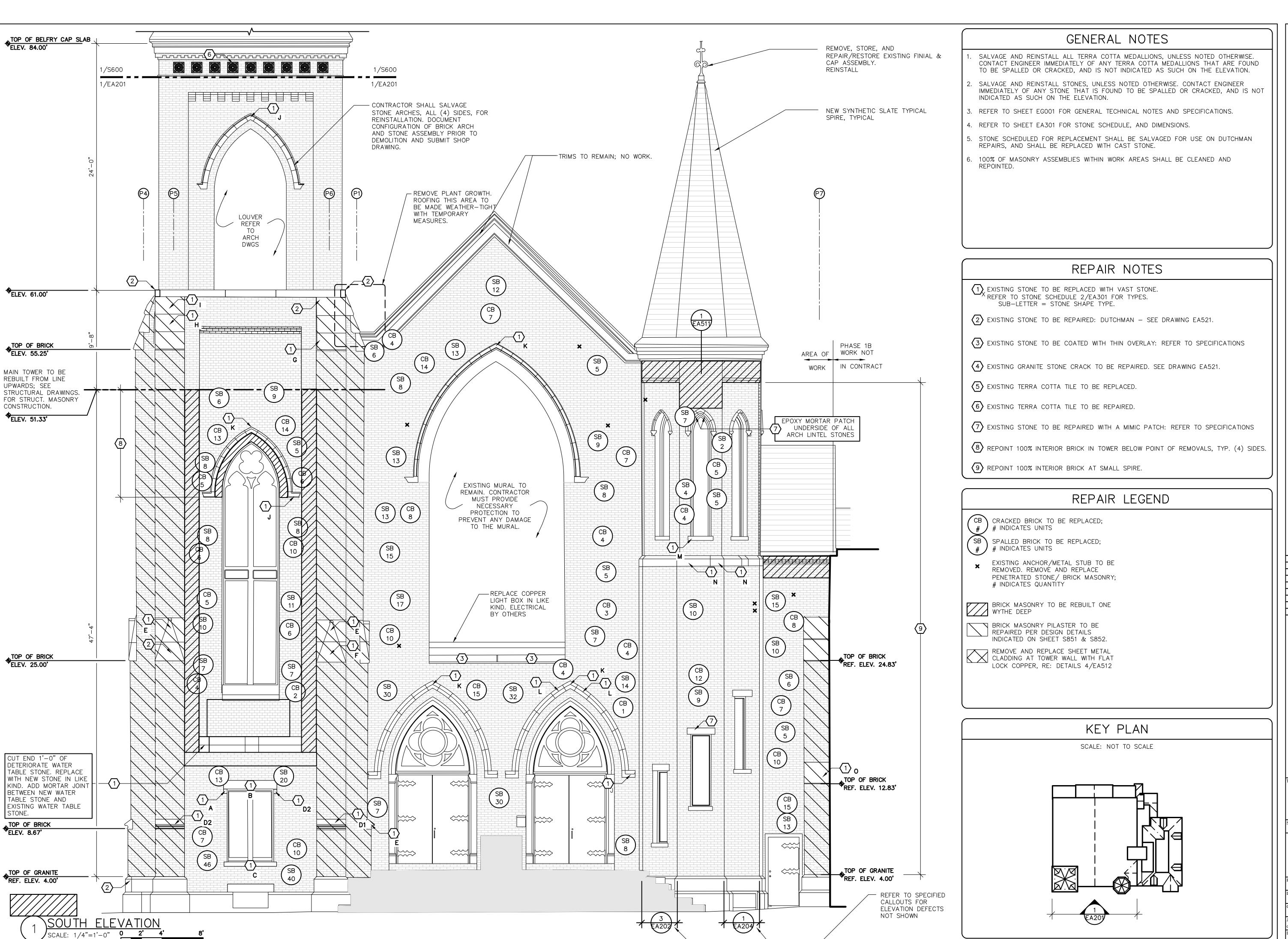
FLOOR PLANS



9/22/2017

Drawn By CAC MDF KPB EWM SMF

Checked By MDF CM Job No. 3704 GALE Job No. 832681 EA101





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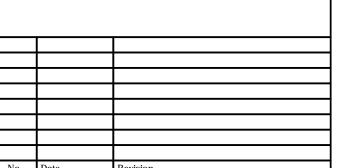
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IN. 000001



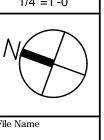
Project:

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Center for the Arts

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PHASE 1A CONSTRUCTION DOCUMENTS

SOUTH ELEVATION

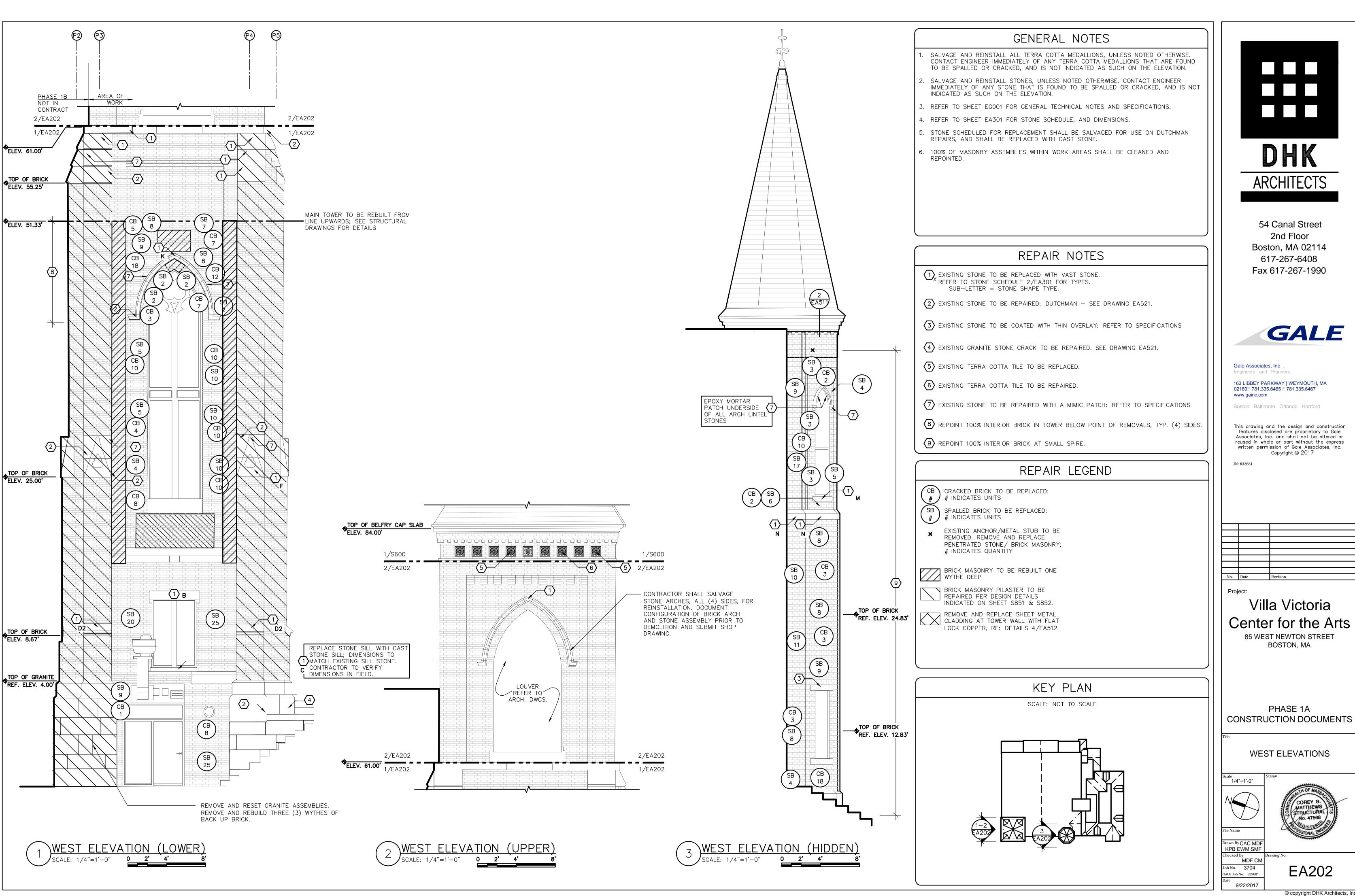


By CAC MDF

Drawn By CAC MDF
KPB EWM SMF
Checked By
MDF CM
Job No. 3704
GALE Job No. 832681

9/22/2017

EA201

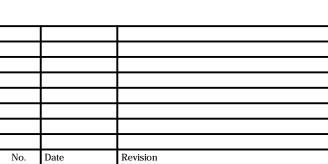




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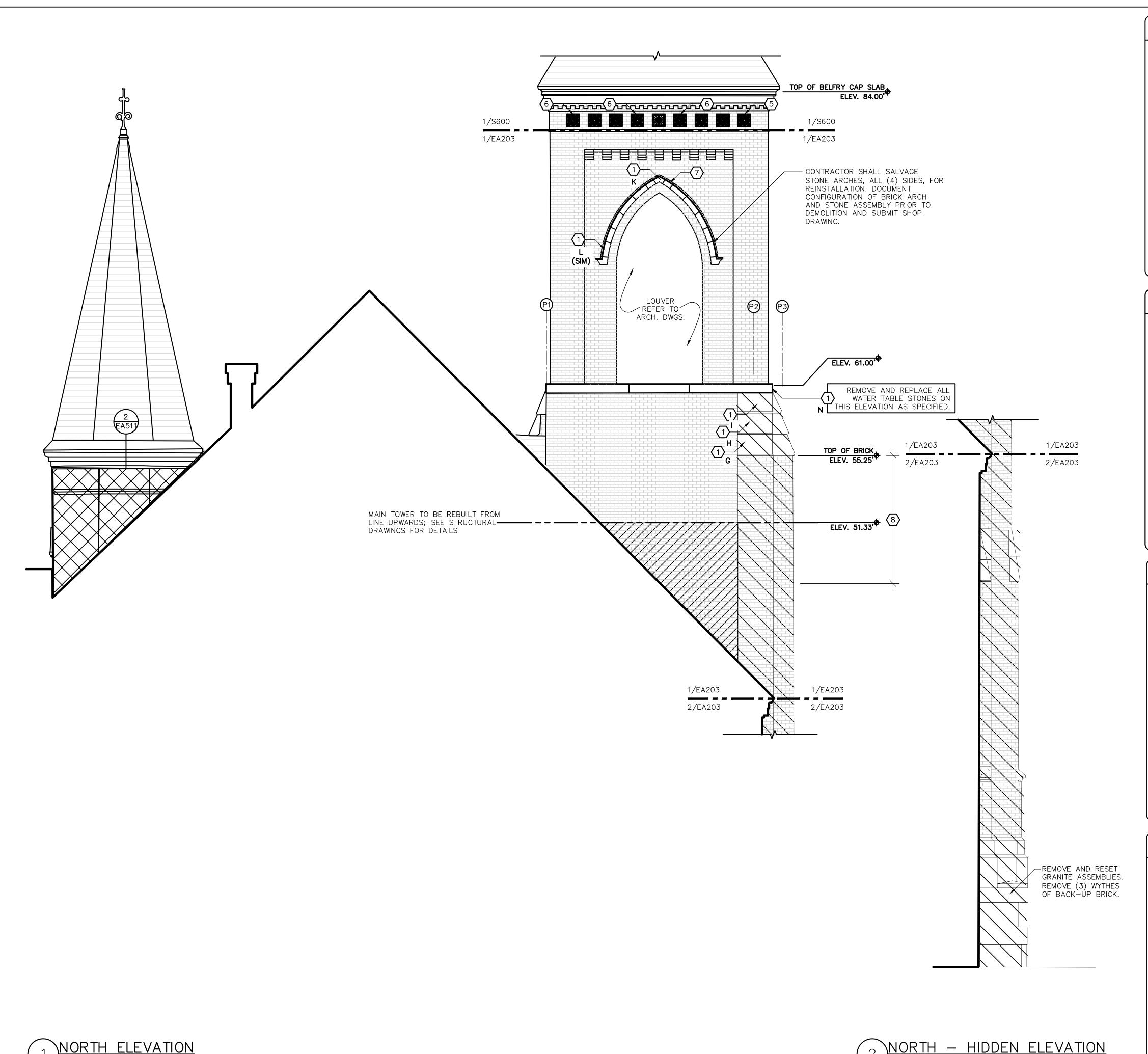
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Villa Victoria

85 WEST NEWTON STREET

CONSTRUCTION DOCUMENTS



GENERAL NOTES

- SALVAGE AND REINSTALL ALL TERRA COTTA MEDALLIONS, UNLESS NOTED OTHERWISE. CONTACT ENGINEER IMMEDIATELY OF ANY TERRA COTTA MEDALLIONS THAT ARE FOUND TO BE SPALLED OR CRACKED, AND IS NOT INDICATED AS SUCH ON THE ELEVATION.
- SALVAGE AND REINSTALL STONES, UNLESS NOTED OTHERWISE. CONTACT ENGINEER IMMEDIATELY OF ANY STONE THAT IS FOUND TO BE SPALLED OR CRACKED, AND IS NOT INDICATED AS SUCH ON THE ELEVATION.
- 3. REFER TO SHEET EGO01 FOR GENERAL TECHNICAL NOTES AND SPECIFICATIONS.
- 4. REFER TO SHEET EA301 FOR STONE SCHEDULE, AND DIMENSIONS.
- 5. STONE SCHEDULED FOR REPLACEMENT SHALL BE SALVAGED FOR USE ON DUTCHMAN REPAIRS, AND SHALL BE REPLACED WITH CAST STONE.
- . 100% OF MASONRY ASSEMBLIES WITHIN WORK AREAS SHALL BE CLEANED AND REPOINTED.

REPAIR NOTES

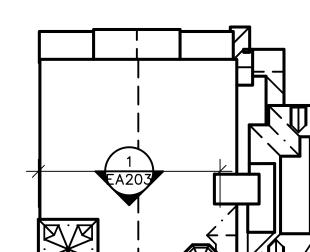
- EXISTING STONE TO BE REPLACED WITH VAST STONE. REFER TO STONE SCHEDULE 2/EA301 FOR TYPES. SUB-LETTER = STONE SHAPE TYPE.
- 2 EXISTING STONE TO BE REPAIRED: DUTCHMAN SEE DRAWING EA521.
- 3 EXISTING STONE TO BE COATED WITH THIN OVERLAY: REFER TO SPECIFICATIONS
- 4 EXISTING GRANITE STONE CRACK TO BE REPAIRED. SEE DRAWING EA521.
- (5) EXISTING TERRA COTTA TILE TO BE REPLACED.
- 6 EXISTING TERRA COTTA TILE TO BE REPAIRED.
- $\overline{7}$ existing stone to be repaired with a mimic patch: refer to specifications
- 8 REPOINT 100% INTERIOR BRICK IN TOWER BELOW POINT OF REMOVALS, TYP. (4) SIDES.
- PREPOINT 100% INTERIOR BRICK AT SMALL SPIRE.

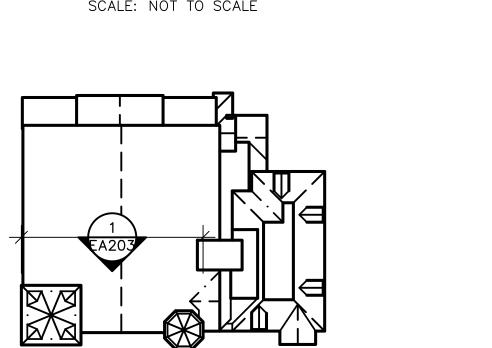
REPAIR LEGEND

- CB CRACKED BRICK TO BE REPLACED; # # INDICATES UNITS
- SB SPALLED BRICK TO BE REPLACED; # # INDICATES UNITS
- EXISTING ANCHOR/METAL STUB TO BE REMOVED. REMOVÉ AND REPLACE PENETRATED STONE / BRICK MASONRY; # INDICATES QUANTITY
- BRICK MASONRY TO BE REBUILT ONE WYTHE DEEP
- BRICK MASONRY PILASTER TO BE REPAIRED PER DESIGN DETAILS INDICATED ON SHEET S851 & S852.
- REMOVE AND REPLACE SHEET METAL CLADDING AT TOWER WALL WITH FLAT LOCK COPPER, RE: DETAILS 4/EA512

KEY PLAN

SCALE: NOT TO SCALE







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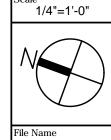
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BOSTON, MA

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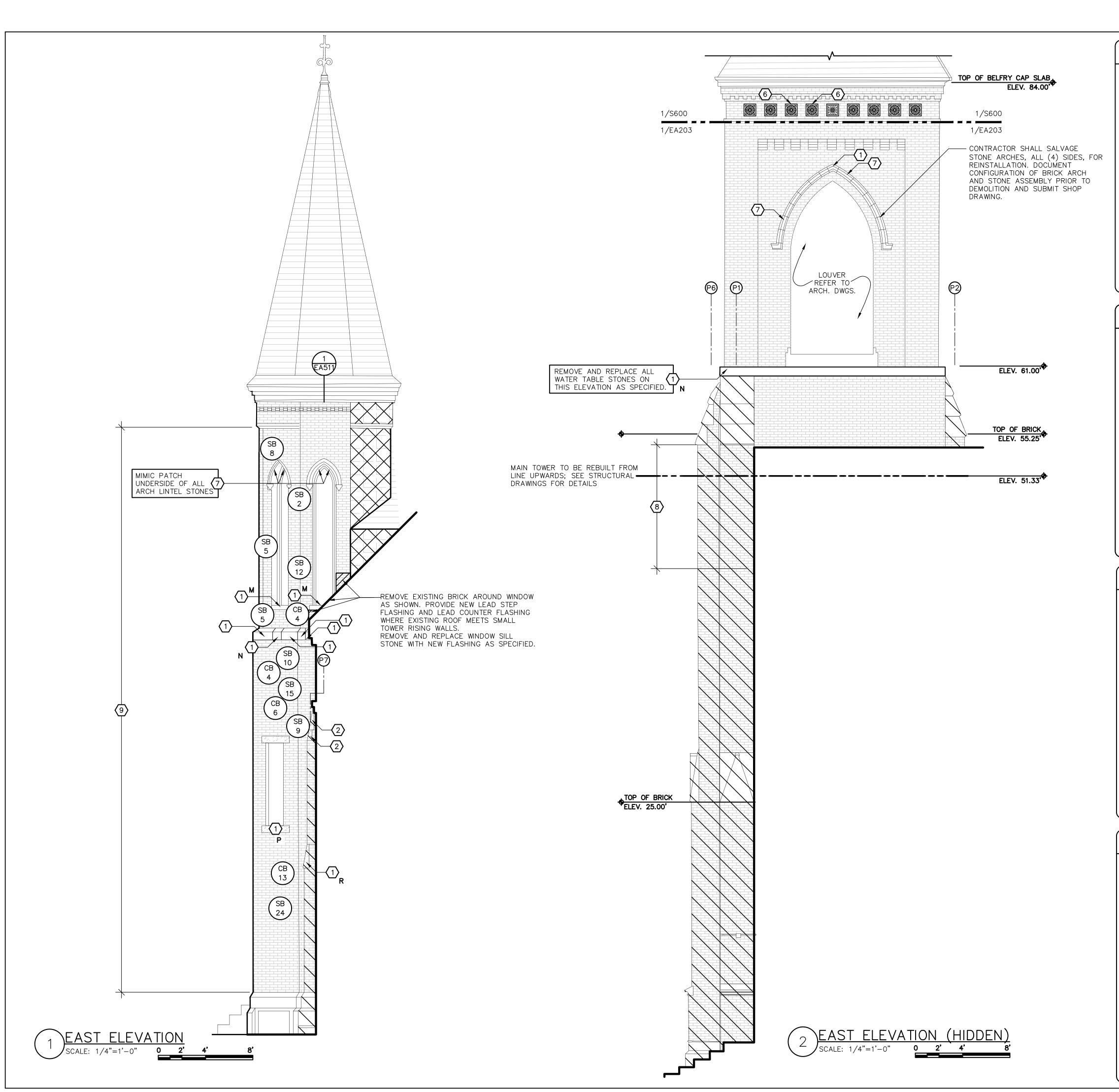
NORTH ELEVATION



Drawn By CAC MDF KPB EWM SMF MDF CM Job No. 3704 GALE Job No. 832681

9/22/2017

EA203



GENERAL NOTES

- SALVAGE AND REINSTALL ALL TERRA COTTA MEDALLIONS, UNLESS NOTED OTHERWISE. CONTACT ENGINEER IMMEDIATELY OF ANY TERRA COTTA MEDALLIONS THAT ARE FOUND TO BE SPALLED OR CRACKED, AND IS NOT INDICATED AS SUCH ON THE ELEVATION.
- SALVAGE AND REINSTALL STONES, UNLESS NOTED OTHERWISE. CONTACT ENGINEER IMMEDIATELY OF ANY STONE THAT IS FOUND TO BE SPALLED OR CRACKED, AND IS NOT INDICATED AS SUCH ON THE ELEVATION.
- 3. REFER TO SHEET EGO01 FOR GENERAL TECHNICAL NOTES AND SPECIFICATIONS.
- REFER TO SHEET EA301 FOR STONE SCHEDULE, AND DIMENSIONS.
- 5. STONE SCHEDULED FOR REPLACEMENT SHALL BE SALVAGED FOR USE ON DUTCHMAN REPAIRS, AND SHALL BE REPLACED WITH CAST STONE.
- . 100% OF MASONRY ASSEMBLIES WITHIN WORK AREAS SHALL BE CLEANED AND REPOINTED.

REPAIR NOTES

- EXISTING STONE TO BE REPLACED WITH VAST STONE. REFER TO STONE SCHEDULE 2/EA301 FOR TYPES. SUB-LETTER = STONE SHAPE TYPE.
- 2 EXISTING STONE TO BE REPAIRED: DUTCHMAN SEE DRAWING EA521.
- 3 EXISTING STONE TO BE COATED WITH THIN OVERLAY: REFER TO SPECIFICATIONS
- 4 EXISTING GRANITE STONE CRACK TO BE REPAIRED. SEE DRAWING EA521.
- 5 EXISTING TERRA COTTA TILE TO BE REPLACED.
- 6 EXISTING TERRA COTTA TILE TO BE REPAIRED.
- (7) Existing stone to be repaired with a mimic patch: refer to specifications
- 8 REPOINT 100% INTERIOR BRICK IN TOWER BELOW POINT OF REMOVALS, TYP. (4) SIDES.
- PREPOINT 100% INTERIOR BRICK AT SMALL SPIRE.

REPAIR LEGEND

- CB CRACKED BRICK TO BE REPLACED; # # INDICATES UNITS
- SB SPALLED BRICK TO BE REPLACED; # # INDICATES UNITS
 - EXISTING ANCHOR/METAL STUB TO BE REMOVED. REMOVÉ AND REPLACE PENETRATED STONE / BRICK MASONRY; # INDICATES QUANTITY

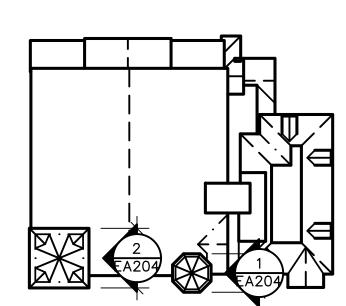
BRICK MASONRY TO BE REBUILT ONE WYTHE DEEP

BRICK MASONRY PILASTER TO BE REPAIRED PER DESIGN DETAILS INDICATED ON SHEET S851 & S852.

REMOVE AND REPLACE SHEET METAL CLADDING AT TOWER WALL WITH FLAT LOCK COPPER, RE: DETAILS 4/EA512

KEY PLAN

SCALE: NOT TO SCALE





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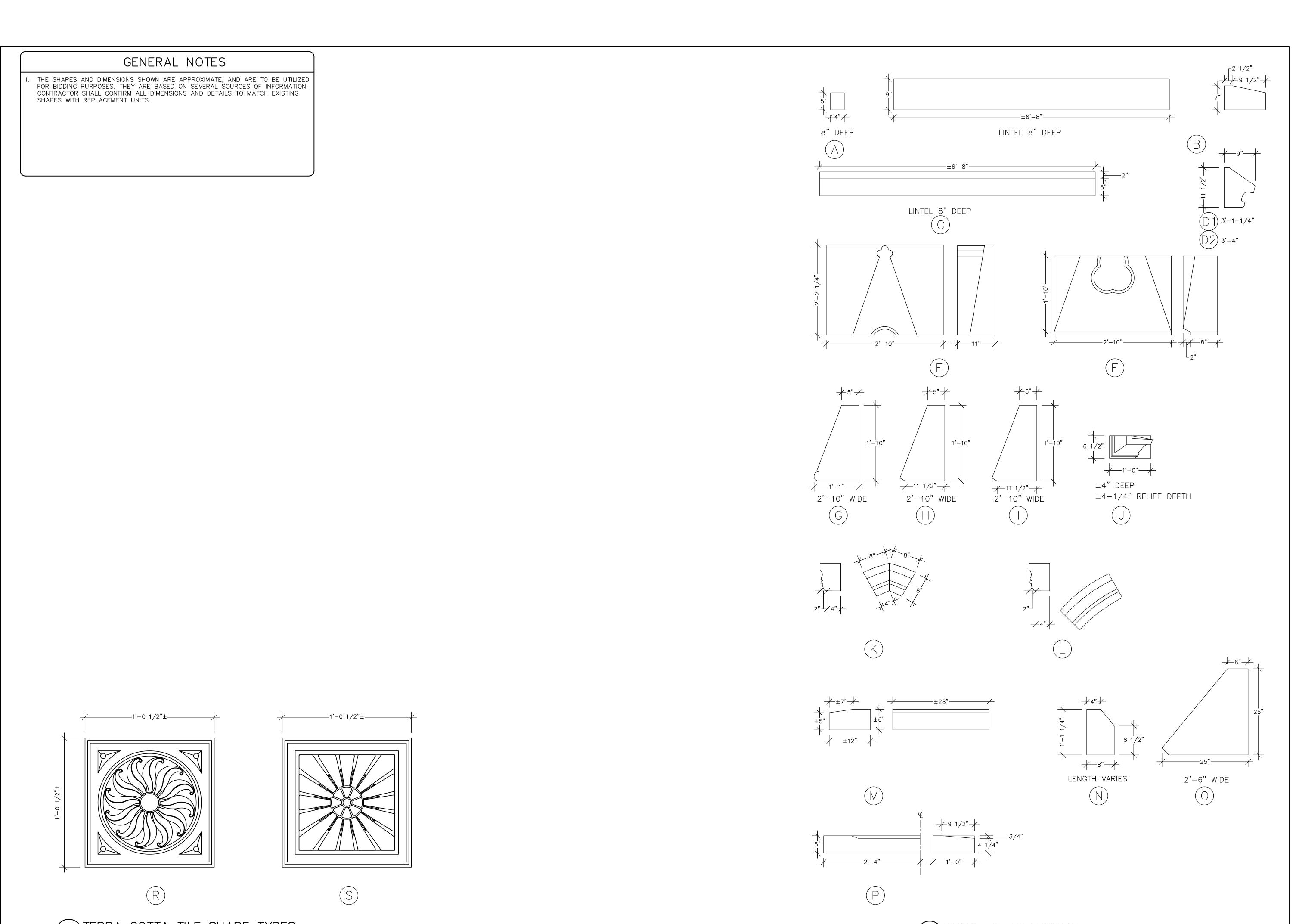
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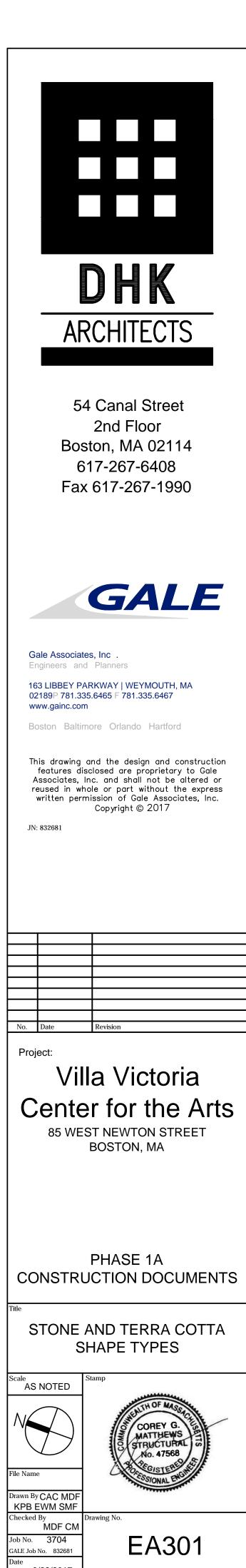
EAST ELEVATIONS

Drawn By CAC MDF KPB EWM SMF MDF CM Job No. 3704 GALE Job No. 832681

9/22/2017

EA204

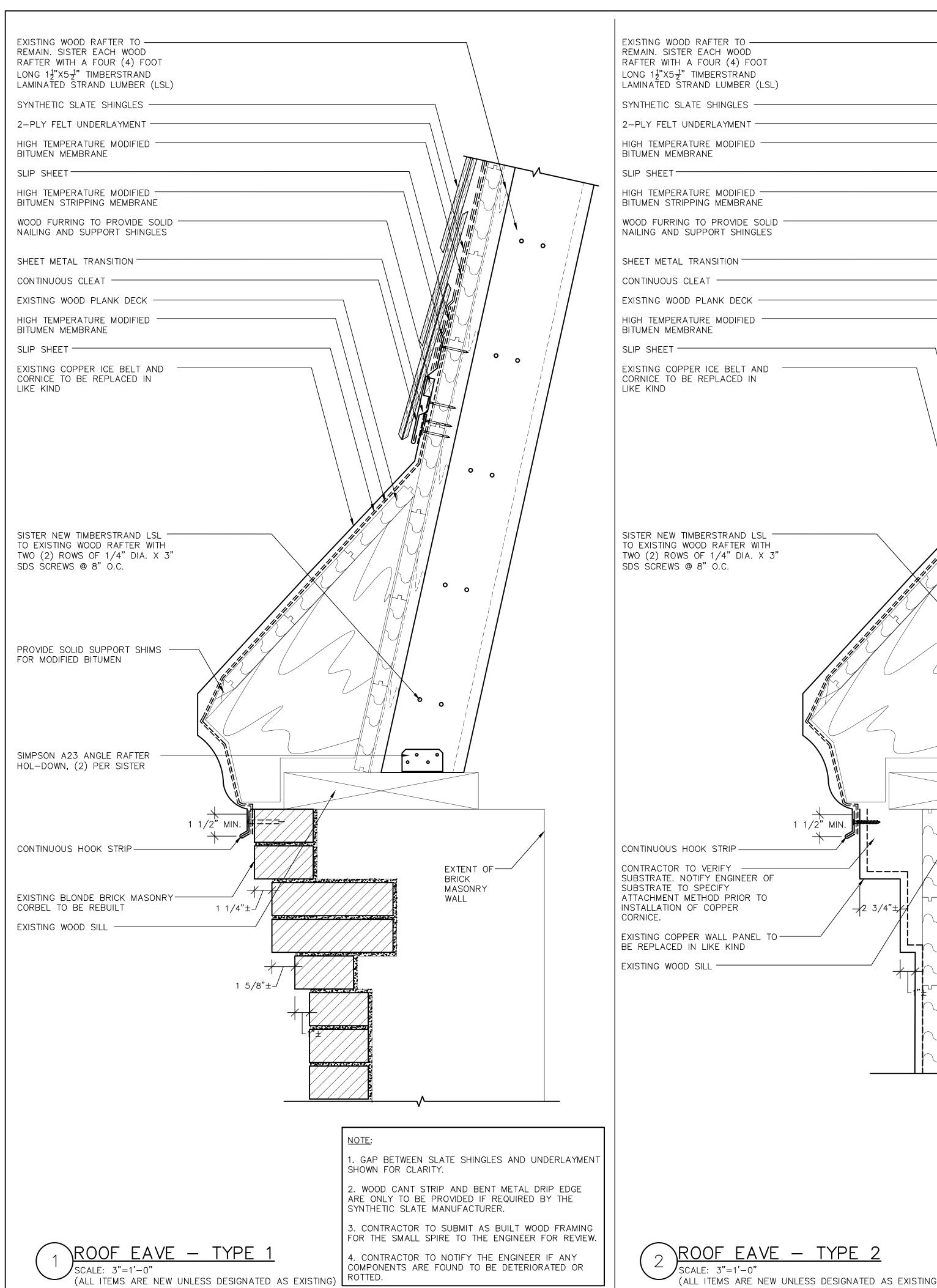


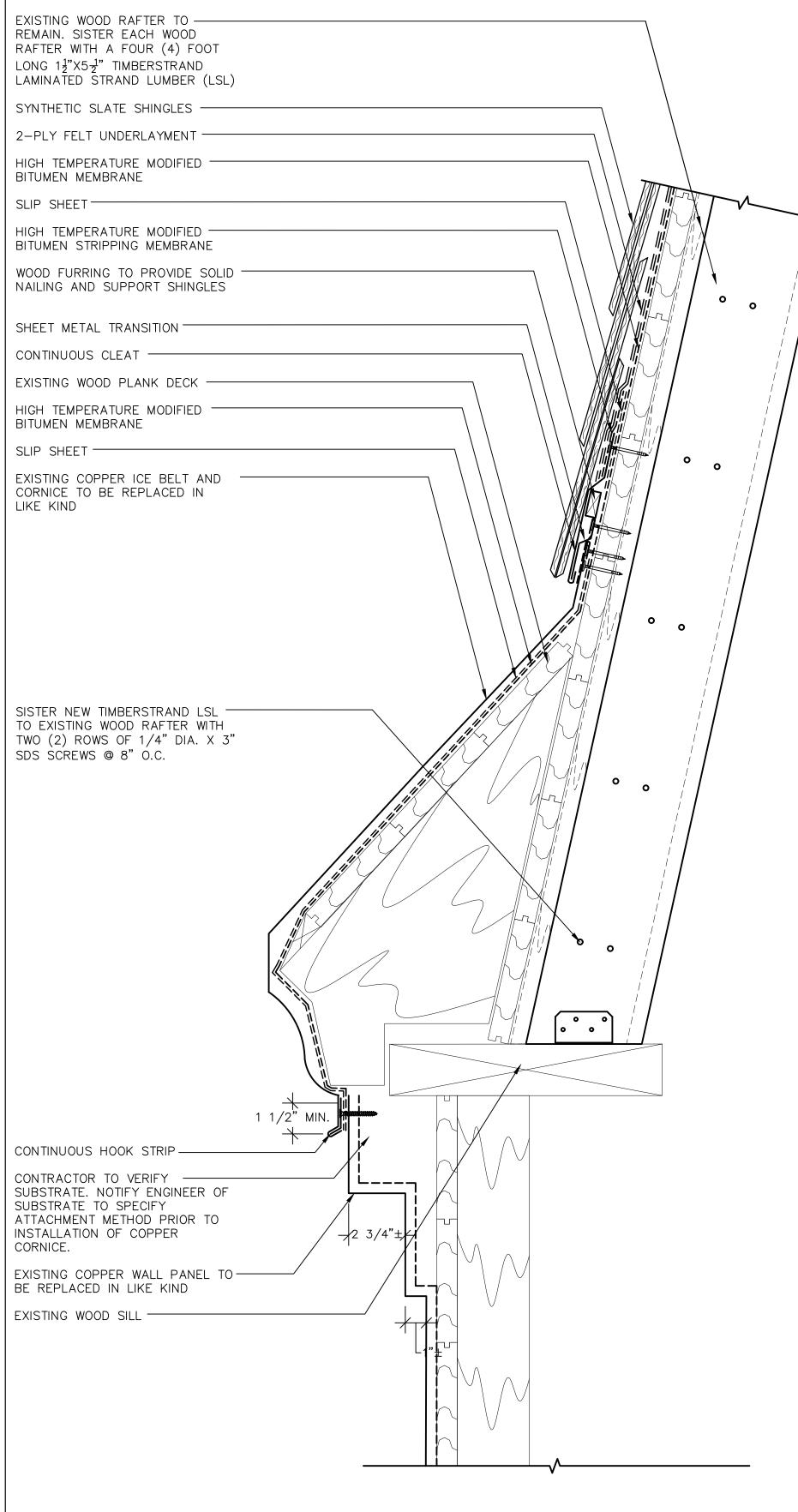


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EA301

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0 3" 6" 9"

. GAP BETWEEN SLATE SHINGLES AND UNDERLAYMENT SHOWN FOR CLARITY.

2. WOOD CANT STRIP AND BENT METAL DRIP EDGE ARE ONLY TO BE PROVIDED IF REQUIRED BY THE SYNTHETIC SLATE MANUFACTURER.

3. CONTRACTOR TO SUBMIT AS BUILT WOOD FRAMING FOR THE SMALL SPIRE TO THE ENGINEER FOR REVIEW.

4. CONTRACTOR TO NOTIFY THE ENGINEER IF ANY COMPONENTS ARE FOUND TO BE DETERIORATED OR



\ROOF EAVE - TYPE 1 - PHOTO



ROOF EAVE — TYPE 1 — PHOTO
SCALE: NTS



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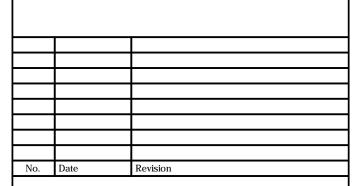


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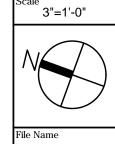


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PHASE 1A CONSTRUCTION DOCUMENTS

SMALL SPIRE ROOF AND **CORNICE DETAILS**

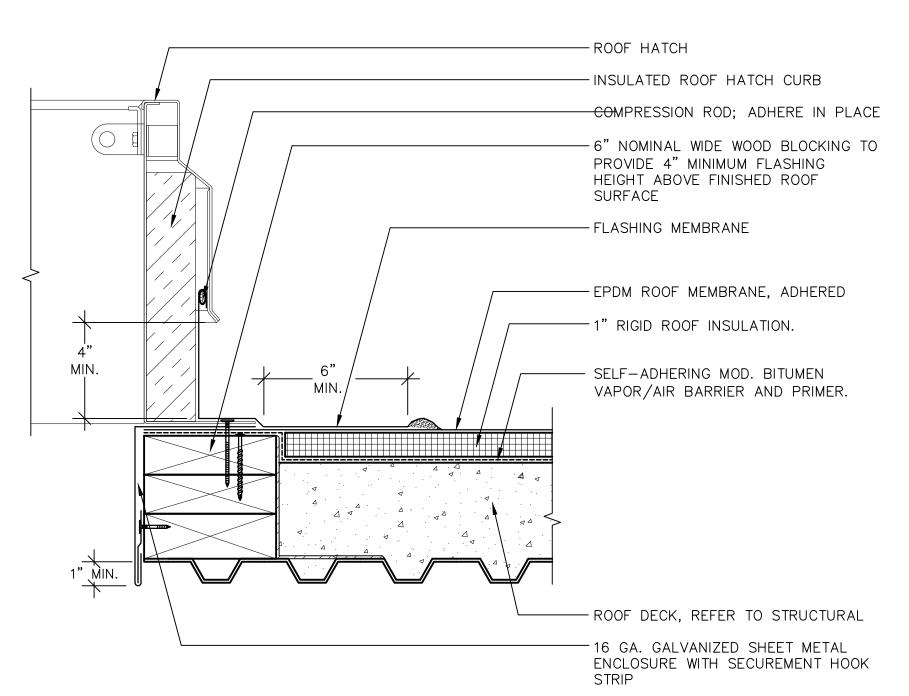


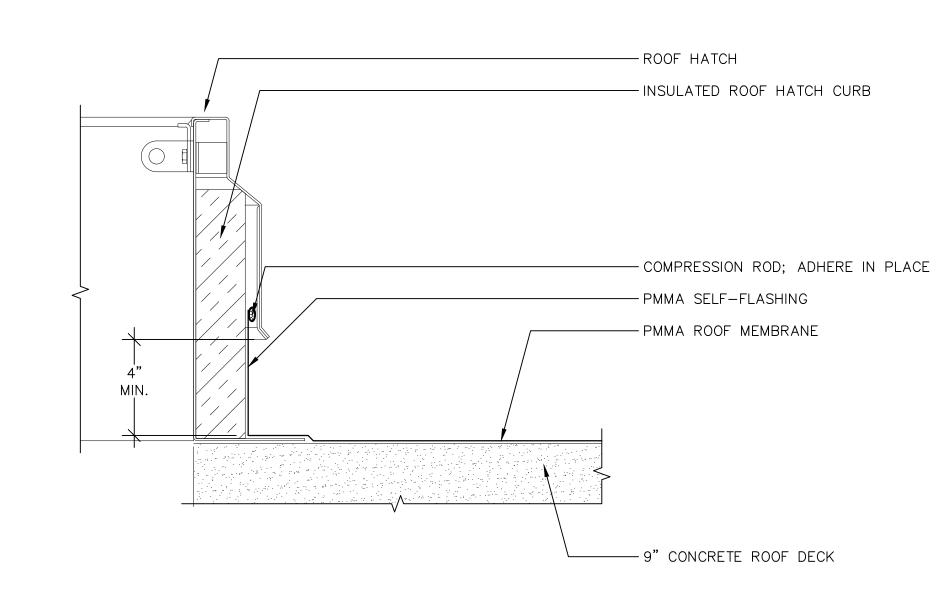
Drawn By CAC MDF KPB EWM SMF Checked By MDF CM GALE Job No. 832681

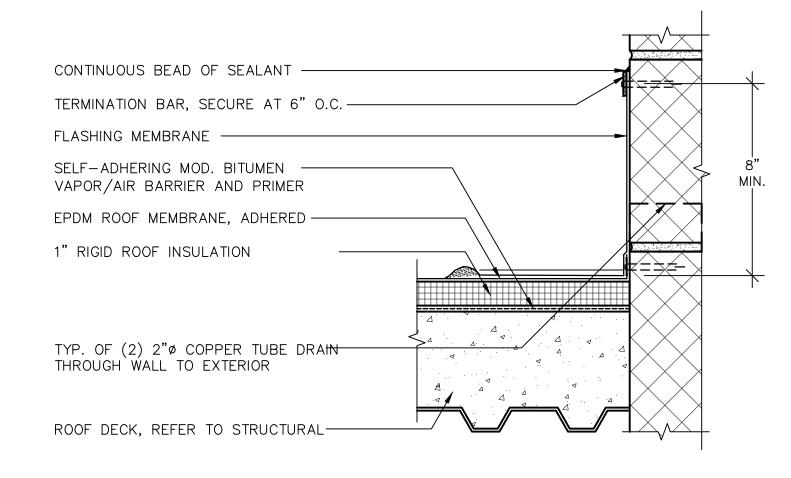
EA511

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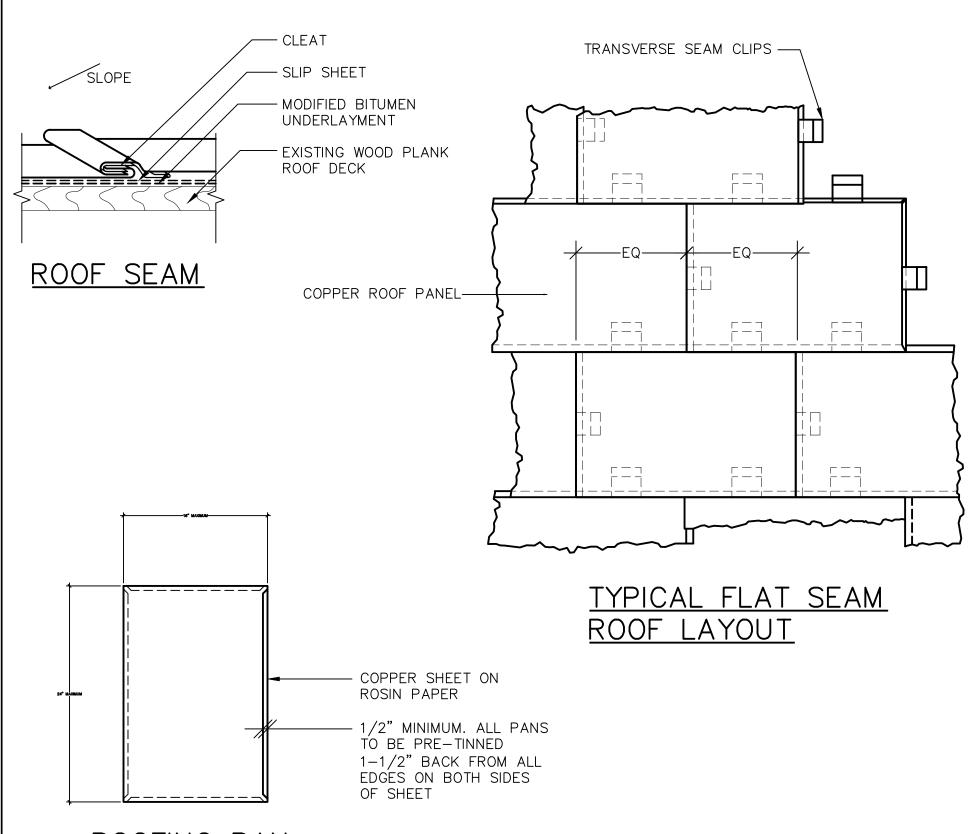




BELFRY FLOOR — ROOF HATCH SCALE: 3"=1'-0" (ALL ITEMS ARE NEW UNLESS DESIGNATED AS EXISTING) 0 3" 6" 9"







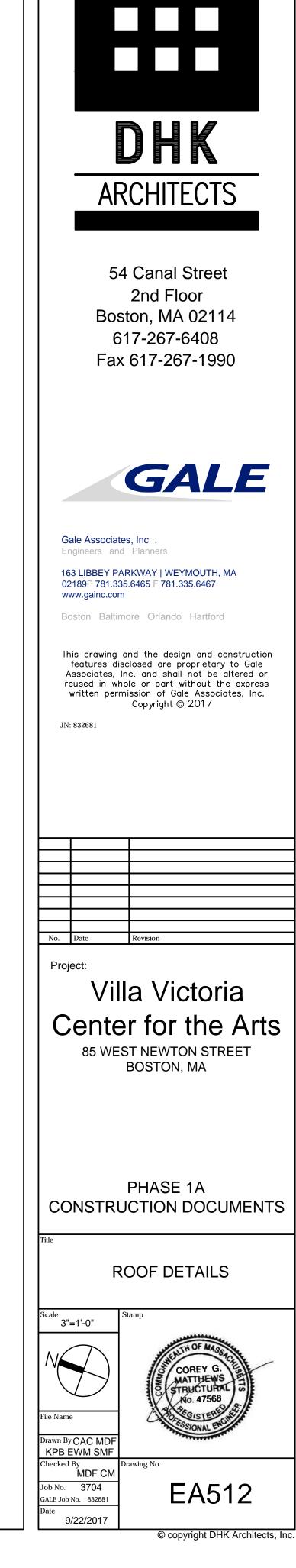
ROOFING PAN

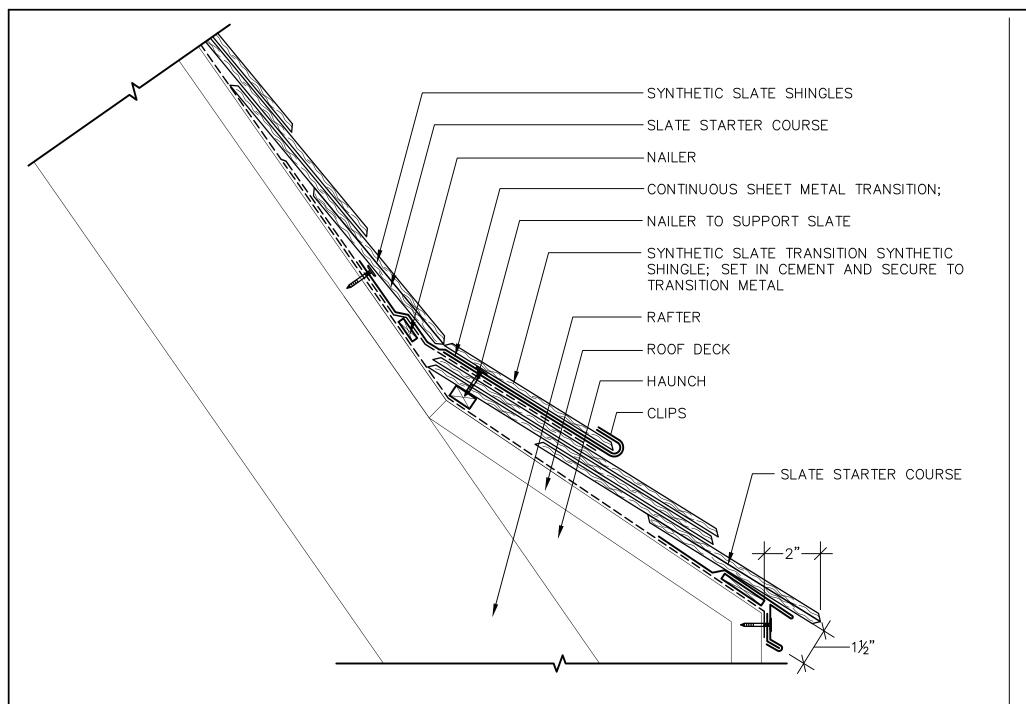
VENTILATOR DETAIL

SCALE: 3"=1'-0"

(ALL ITEMS ARE NEW UNLESS DESIGNATED AS EXISTING)

0 3" 6" 9"





- MODIFIED BITUMEN STRIPPING MEMBRANE NOTE: "E" REFERS TO THE EXPOSURE OF THE SLATE. REFER TO THE SLATE ROOF CONFIGURATION DETAIL ON SHEET EA513 FOR THE TYPICAL SLATE EXPOSURE. MODIFIED BITUMEN FLASHING -STEPPED SHEET METAL FLASHING SYNTHETIC SLATE SHINGLES; -CUT TO PROVIDE CLOSED VALLEY; ALL SHINGLES TO HAVE A MINIMUM OF TWO (2) **FASTENERS**

SCALE: NOT TO SCALE

TYPICAL HEADLAP (AMOUNT OF SHINGLE COVERED BY SHINGLE ABOVE)

- ROOF EDGE

- STARTER COURSE AT EAVE

W=12"____

SYNTHETIC SLATE CONFIGURATION AT EDGE

(ALL ITEMS ARE NEW UNLESS DESIGNATED AS EXISTING)

TYPICAL FIELD SLATE

HEAD LAPS FOR SYNTHETIC SLATE SHINGLE

REPLACEMENT FOR THE STEEP SLOPE SPIRE SHALL BE 3" MINIMUM UNLESS REQUIRED

OTHERWISE BY MANUFACTURER

TYPICAL DIMENSIONS SLATE LENGTH SLATE EXPOSURE 7.5" CONFIRM IN FIELD

> EXPOSURE LEGEND

L - LENGTH OF SLATE SHINGLE

E - EXPOSURE OF SLATE

H - HEADLAP OF SLATE

−H=2"

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BOSTON, MA

CONSTRUCTION DOCUMENTS

PHASE 1A

TYPICAL SYNTHETIC SLATE **ROOF DETAILS**

AS NOTED

Drawn By CAC MDF KPB EWM SMF Checked By MDF CM

9/22/2017

EA513 GALE Job No. 832681

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. GAP BETWEEN SLATE SHINGLES AND UNDERLAYMENT SHOWN FOR CLARITY.

2. WOOD CANT STRIP AND BENT METAL DRIP EDGE ARE ONLY TO BE PROVIDED IF REQUIRED BY THE SYNTHETIC SLATE MANUFACTURER.

CLOSED VALLEY

SCALE: NOT TO SCALE (ALL ITEMS ARE NEW UNLESS DESIGNATED AS EXISTING)

. GAP BETWEEN SLATE SHINGLES AND UNDERLAYMENT

ROOF CEMENT; POINT EACH ----

SHOWN FOR CLARITY. 2. WOOD CANT STRIP AND BENT METAL DRIP EDGE ARE ONLY TO BE PROVIDED IF REQUIRED BY THE SYNTHETIC SLATE MANUFACTURER.

FILL GAP OR CHAMFER BLOCKING TO PROVIDE SOLID -SYNTHETIC SLATE SHINGLES; CUT AS REQUIRED FOR — STEPPED SHEET METAL FLASHING WOVEN INTO SHINGLES MODIFIED BITUMEN UNDERLAYMENT; EXTEND 12" MIN.

SECTION VIEW

SLATE HIP JOINT SYNTHETIC SLATE; CUT AS — REQUIRED FOR MITERED HIP CONSTRUCTION STEPPED SHEET METAL -FLASHING WOVEN INTO SHINGLES 1 1/2" MIN. HIP NAIL -DISTANCE FROM BUTT EDGE OF THE NEXT COURSE SPOT OF MASTIC TO SET EACH HIP SLATE **ELEVATION VIEW**

. GAP BETWEEN SLATE SHINGLES AND UNDERLAYMENT SHOWN FOR CLARITY.

2. WOOD CANT STRIP AND BENT METAL DRIP EDGE ARE ONLY TO BE PROVIDED IF REQUIRED BY THE SYNTHETIC SLATE MANUFACTURER.

(ALL ITEMS ARE NEW UNLESS DESIGNATED AS EXISTING)

SCALE: 3"=1'-0"

MITERED SLATE HIP

DORMER EDGE SOFFIT

3" 6" 9"

(ALL ITEMS ARE NEW UNLESS DESIGNATED AS EXISTING)

SCALE: 3''=1'-0''

SLATER CEMENT ----

MITERED HIP

ROOF DECK -

2-PLY UNDERLAYMENT-

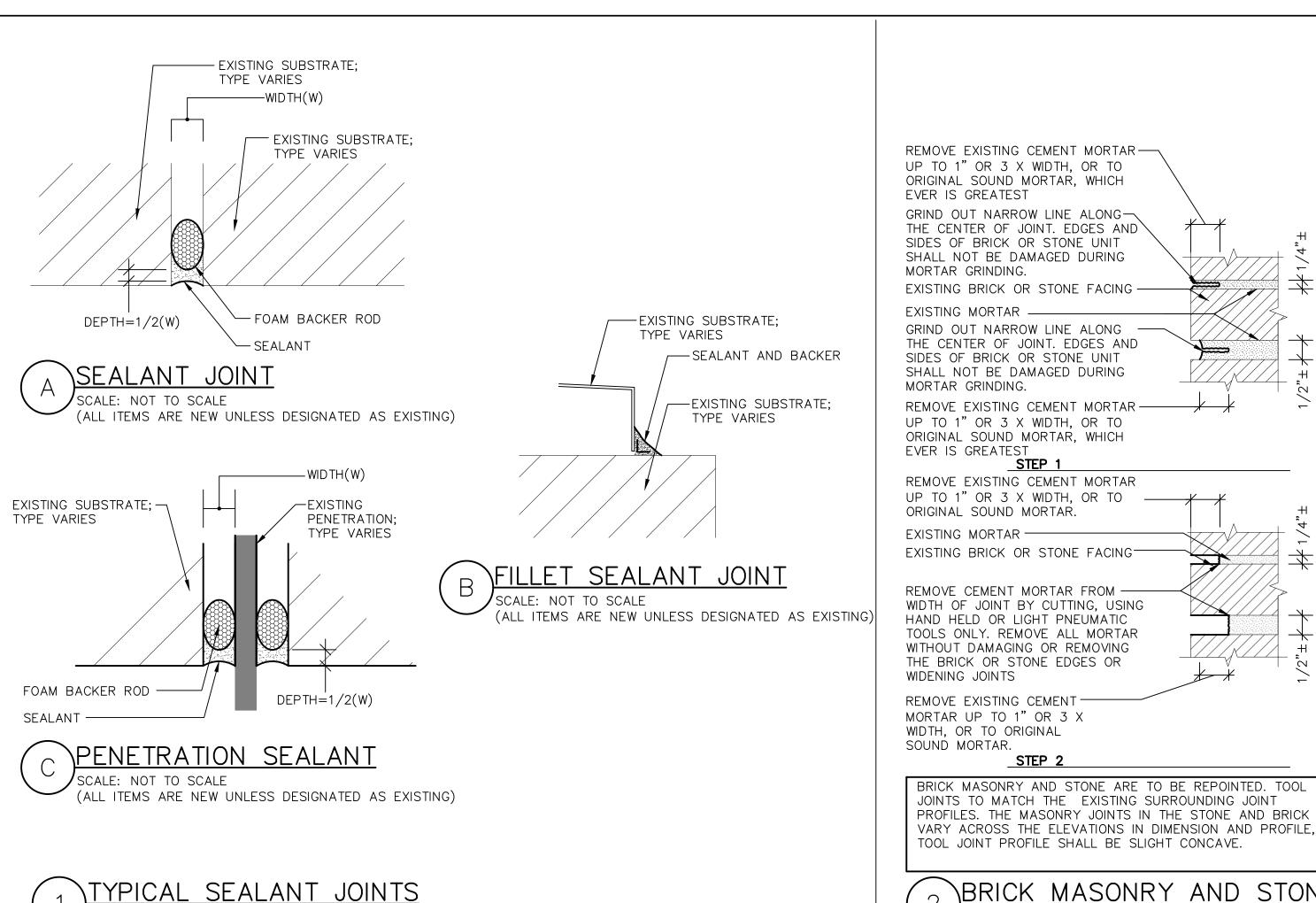
RAFTER

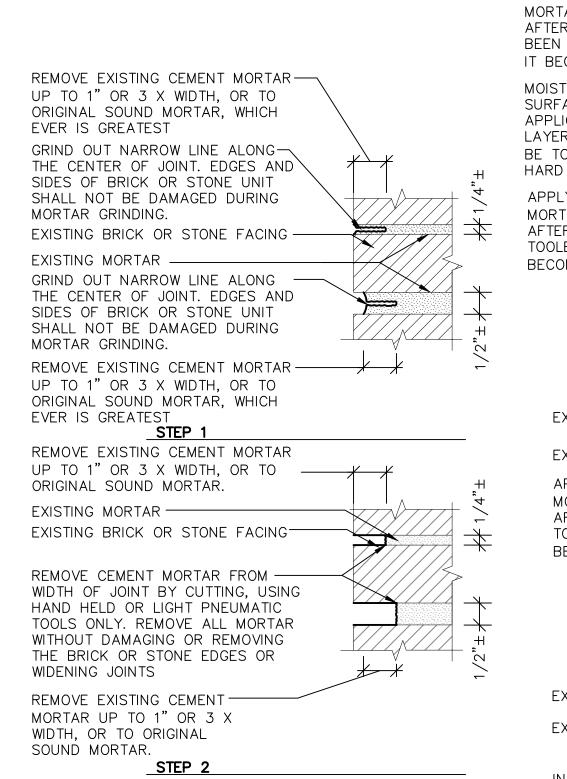
HIP BOARD -

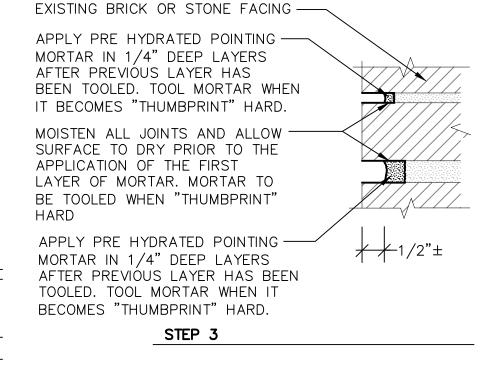
BEARING FOR UNDERLAYMENT

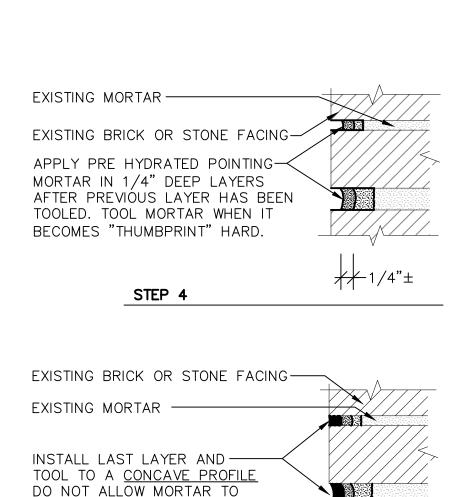
BLOCKING TO SUPPORT SLATE -

BEYOND HIP IN EACH DIRECTION









COME INTO CONTACT WITH OR

RESIDUE TO REMAIN ON ANY

EXPOSED BRICK OR STONE

GENERAL PROCEDURES FOR DUTCHMAN STONE REPAIR

SUBSTRATE.

SURFACE.

THE NEW STONE.

. CUT OUT AREA OF SPALL TO CLEAN, SOUND

2. THE CUT OUT AREA SHOULD BE RECTILINEAR;

STONE BLOCK AND PERPENDICULAR TO STONE

WITH THE SIDES PARALLEL TO THE EDGES OF THE

3. MEASURE AND CUT NEW STONE TO MATCH THE

REPAIR AREA WITH MINIMUM SIZED JOINTS AROUND

4. INSTALL ANCHORS AS REQUIRED DEPENDING ON

SIZE OF THE DUTCHMAN. REFER TO TABLE BELOW.

ANCHORS SHALL BE STAINLESS STEEL DOWELS OR

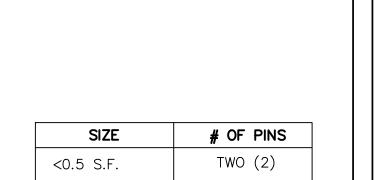
5. INSTALL DUTCHMAN; ADHERE WITH EPOXY RESIN

EXISTING STONE

— AREA OF SPALL

CRAMPS AND SHALL BE SET IN EPOXY RESIN.

OR POLYMER MODIFIED CEMENT GROUT.



FOUR (4)

SIX (6)

— EXISTING STONE

CUT OUT SPALL AREA;

CUTS ARE AT 90° ANGLES

- EXISTING STONE

STONE

0.5-1.0 S.F.

1.0-2.0 S.F.

- GROUT AT PERIMETER OF

TO MATCH EXISTING STONE

— STAINLESS STEEL ANCHORS SET

COLOR, AGGREGATE SIZE AND

TOOLING OF THE SURROUNDING

IN EPOXY RESIN OR GROUT

-- DUTCHMAN TO MATCH THE

DUTCHMAN; PROVIDE SURFACING

PREDRILLED

PREDRILLED

ANCHOR HOLE

ANCHOR HOLE

BRICK MASONRY AND STONE REPOINTING SCALE: 3"=1'-0" (ALL ITEMS ARE NEW UNLESS DESIGNATED AS EXISTING)



(REFER TO SPECIFICATIONS FOR ADDITIONAL INFORMATION)

3" 6" 9"

WIDEN CRACKS AS REQUIRED BY CONTINUOUS SAWCUT ROUTING. CLEAN

2. SPACE PORTING DEVICES AS REQUIRED BASED ON DEPTH AND WIDTH OF CRACK. DO NOT SPACE GREATER THAN 6" APART. OR D/2.

MASK EDGES OF CRACK TO CONTROL WIDTH OF APPLIED CAP SEAL.

4. INSERT PORTING DEVICES.

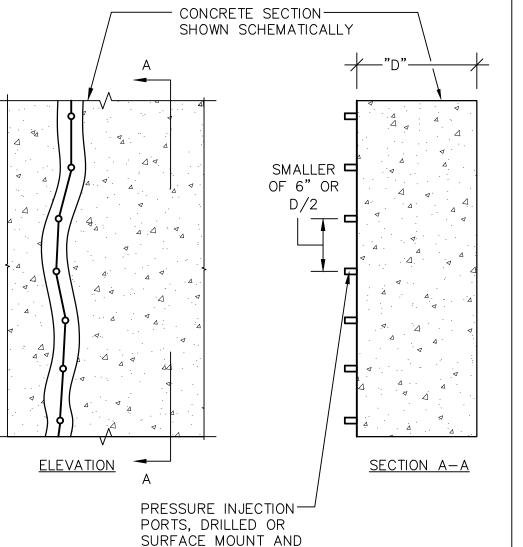
APPLY EPOXY CAP SEAL CONTINUOUSLY ALONG CRACK AND AROUND PORTS. CLEAR PORTS TO ENSURE EPOXY HAS NOT BLOCKED PASSAGE.

AFTER CAP SEAL CURE, USE HIGH-PRESSURE INJECTION EQUIPMENT TO INJECT APPROPRIATE VISCOSITY EPOXY INTO PORTS. START AT BOTTOM PORT, FILLING FULL DEPTH OF CRACK. DO NOT MOVE TO NEXT PORT UNTIL EPOXY BEGINS TO FLOW FROM ADJACENT PORT, THUS INDICATING TRAVEL. EXTEND EPOXY WITH SAND AS SPECIFIED, IF REQUIRED.

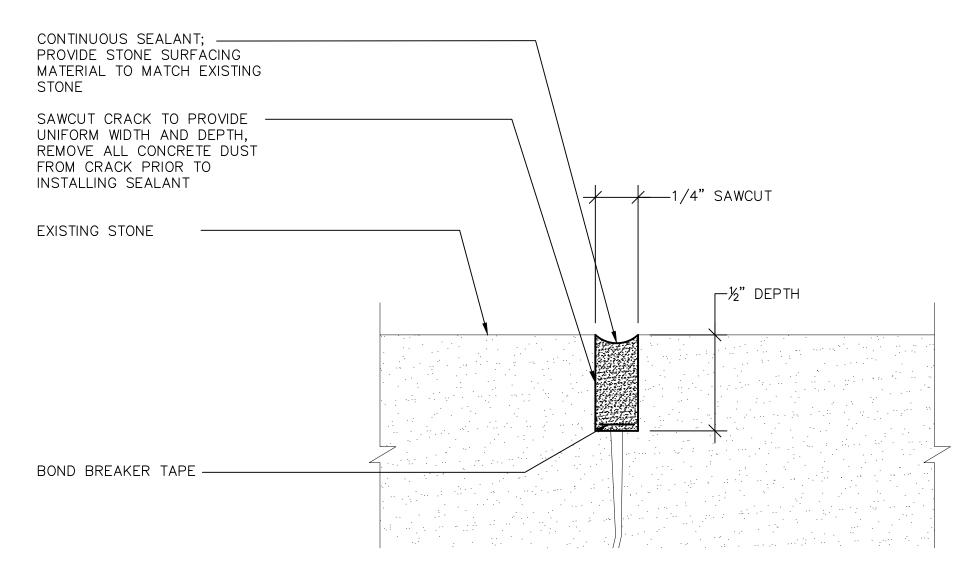
AFTER ENTIRE CRACK IS FILLED AND EPOXY CURED, REMOVE PORTS AND GRIND CAP SEAL FLUSH.

9. QUANTITY OF PORTS SHOWN SCHEMATICALLY.

NOTE: COLOR OF CAP SEAL TO MATCH EXISTING STONE AS CLOSELY AS POSSIBLE. COLLECT STONE DUST AND APPLY TO SURFACE OF CAP SEAL TO BLEND REPAIR.

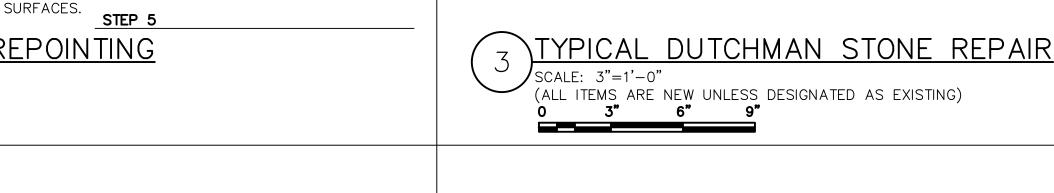


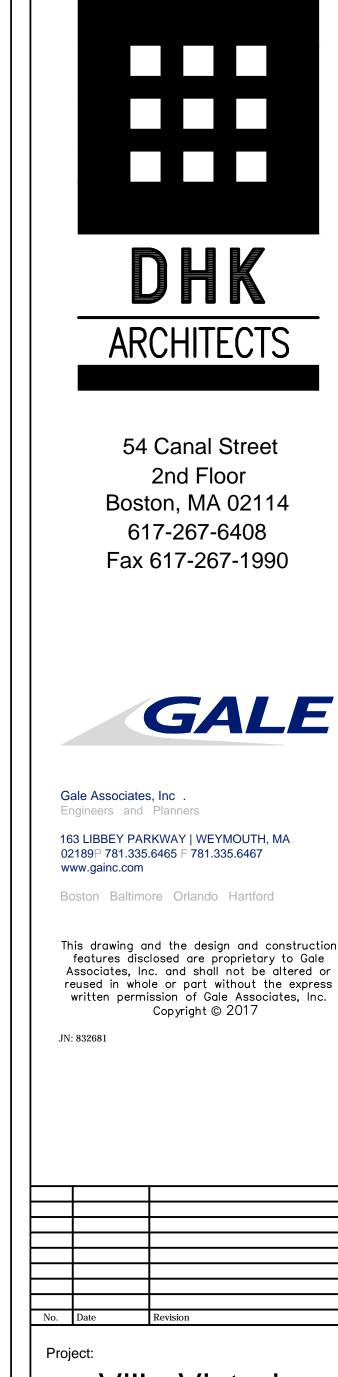
EPOXIED IN PLACE

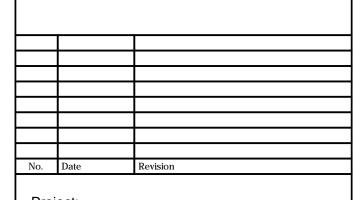


STONE CRACK (WIDTH >1/16"): \TYPICAL CRACK PRESSURE INJECTION EPOXY REPAIR

STONE CRACK (<1/16"): \TYPICAL STONE CRACK — ROUT & SEAL REPAIR SCALE: NOT TO SCALE (ALL ITEMS ARE NEW UNLESS DESIGNATED AS EXISTING)





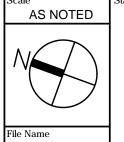


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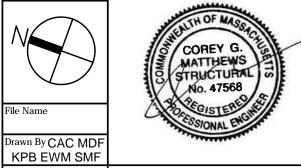
PHASE 1A CONSTRUCTION DOCUMENTS

TYPICAL BUILDING REPAIR **DETAILS**



9/22/2017

Checked By



MDF CM Job No. 3704 **EA521** GALE Job No. 832681

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TYPICAL SPALL PATCHING DETAILS (ALL ITEMS ARE NEW UNLESS DESIGNATED AS EXISTING)

SCALE: NOT TO SCALE

GENERAL PROCEDURES FOR SURFACE DEFECT

CUT OUT AREA OF SPALL TO CLEAN, SOUND

LESS THAN TWO (2) INCHES, MECHANICAL

BLOCK AND PERPENDICULAR TO STONE

FOR AN IMPROVED MECHANICAL BOND.

THE SUBSTRATE TO RECEIVE PATCH.

THE SURROUNDING STONE. REFER TO

MECHANICAL AND CHEMICAL BONDING;

APPLY FINISH COAT TO MATCH TEXTURE,

SHOULD WEATHER IN SAME MANNER AS

GENERAL PROCEDURES FOR DEEP SPALL REPAIR

BACK-ANGLE SAWCUT PERIMETER - POLYGON

EXAMINE SUBSTRATE FOR CRACKS. REPORT TO

SCARIFY SUBSTRATE TO 1/8" MINIMUM SURFACE

CURE PATCH AS RECOMMENDED BY

CHIP OUT ALL DETERIORATED STONE.

PROFILE (ICRI - CSP 8 OR 9). WASH

APPLY EPOXY MODIFIED CEMENTITIOUS

PRIMER / BOND AGENT TO ALL STONE

BONDING AGENT TO ACHIEVE A

SECTION SHOWN SCHEMATICALLY.

"WET-ON-WET" APPLICATION.

SURFACE DRY) CONDITION.

SUBSTRATE (1 COAT).

SUBSTRATE AND LEAVE SSD (SATURATED

APPLY SPECIFIED REPAIR MORTAR AFTER

3" 6" 9"

COLOR, AND TOOLING OF THE SURROUNDING

STONE AT DESIGNATED LOCATIONS. PATCH

MANUFACTURERS INSTRUCTIONS FOR

SURFACE.

DOVETAIL.

RESPECTFULLY.

ORIGINAL STONE.

MANUFACTURER.

ENGINEER

ANCHORS MAY NOT BE REQUIRED. THE CUT

SUBSTRATE. FOR A SPALL WITH A DEPTH OF

OUT AREA SHOULD BE RECTILINEAR; WITH THE

SLIGHTLY UNDERCUT THE SIDES TO CREATE A

DRILL HOLES AT THE BACK WALL OF THE CUT

APPLY SLURRY COAT TO ENTIRE SURFACE OF

SUBSTRATE SHOULD BE SATURATED, SURFACE

INSTALL SCRATCH COATS OF COMPOSITE PATCH

MATERIAL IN LAYERS UNTIL THE PATCH IS

BUILT UP TO JUST BELOW THE SURFACE OF

APPLICATION THICKNESSES AND CURING TIMES.

SCRATCHED AND WETTED TO ENSURE PROPER

SURFACE OF EACH OF THE LAYERS SHOULD BE

SIDES PARALLEL TO THE EDGES OF THE STONE

(ALL ITEMS ARE NEW UNLESS DESIGNATED AS EXISTING)

■ MIN. EXISTING STONE - CEMENTITIOUS REPAIR MORTAR (AS SPECIFIED). 2" X 2" X 12 GA HDG OR SS WELDED WIRE MESH TIED TO THRU-HOLES OF EACH INSERTED PIN. MESH REINF SHALL COVER ENTIRE SURFACE AREA OF REPAIR AT EACH 13" OF DEPTH. DRILL & EPOXY (OR MECH (WITH OUTER END THRU-HOLE) INTO SUBSTRATE OF REPAIR AREA. MAINTAIN 2" MIN. EDGE DISTANCE FOR EACH PIN. MORTAR (TYP.) - EPOXY MODIFIED CEMENTITIOUS

SECTION VIEW SPALL DEPTH < 2"

FASTEN) 1/4" MIN. DIA. SS PINS BACK-CUT AROUND PERIMETERS OF SPALL, TO "LOCK-IN" REPAIR

- EXISTING STONE

STRENGTH

— SLURRY COAT

STONE

- PREDRILLED HOLES FOR

ADDITIONAL MECHANICAL

SCRATCH COAT: LAYERED

APPLICATION (%" LAYER

- FINISH COAT; MATCH SURFACE

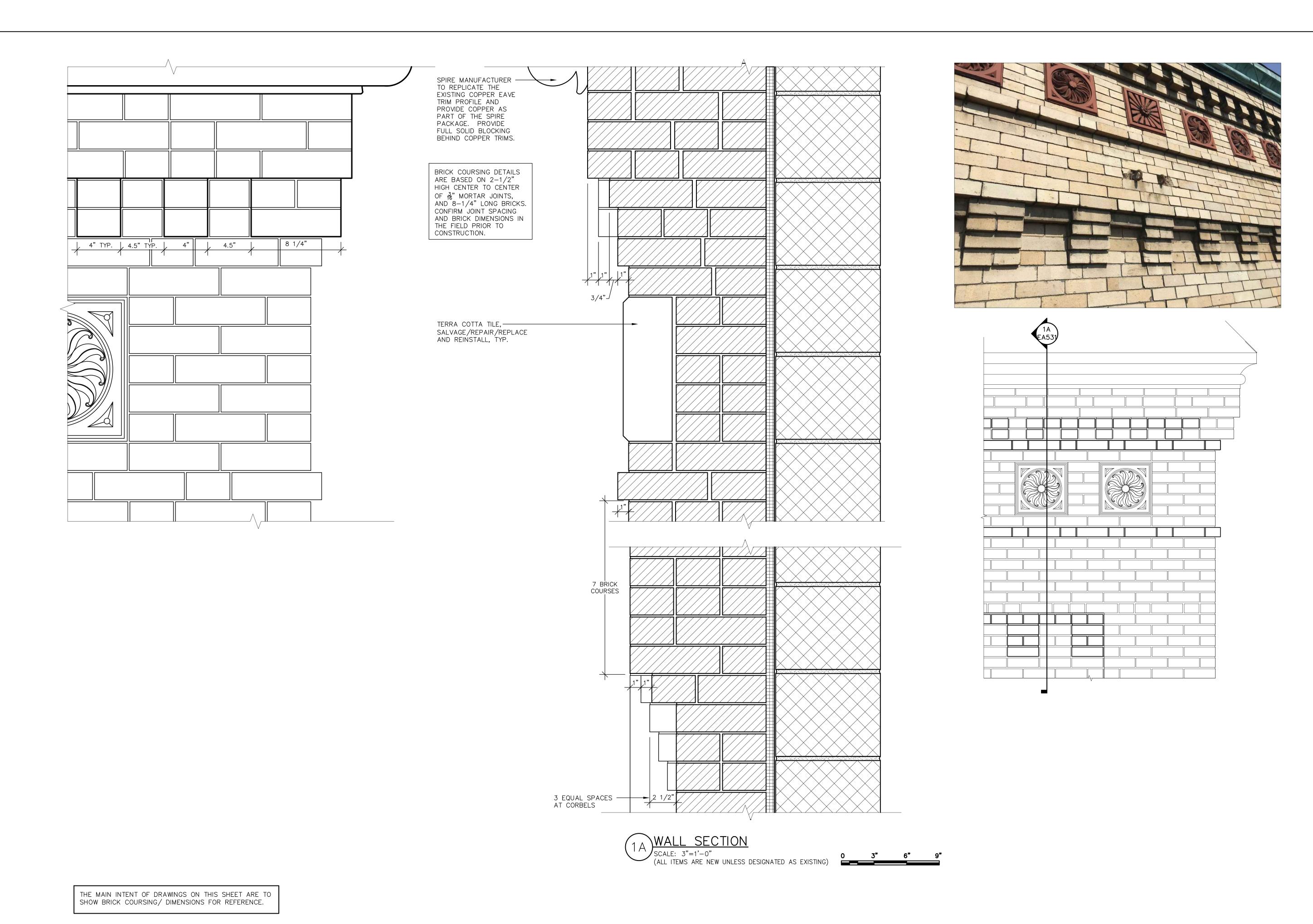
AND COLOR OF SURROUNDING

BONDING AGENT (AS SPECIFIED) ON PREPARED SUBSTRATE.

4B SECTION VIEW SPALL DEPTH > 2"

3" 6" 9"

(ALL ITEMS ARE NEW UNLESS DESIGNATED AS EXISTING)







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PHASE 1A CONSTRUCTION DOCUMENTS

MASONRY DETAILS

Drawn By CAC MDF
KPB EWM SMF
Checked By
MDF CM
Job No. 3704
GALE Job No. 832681
Date

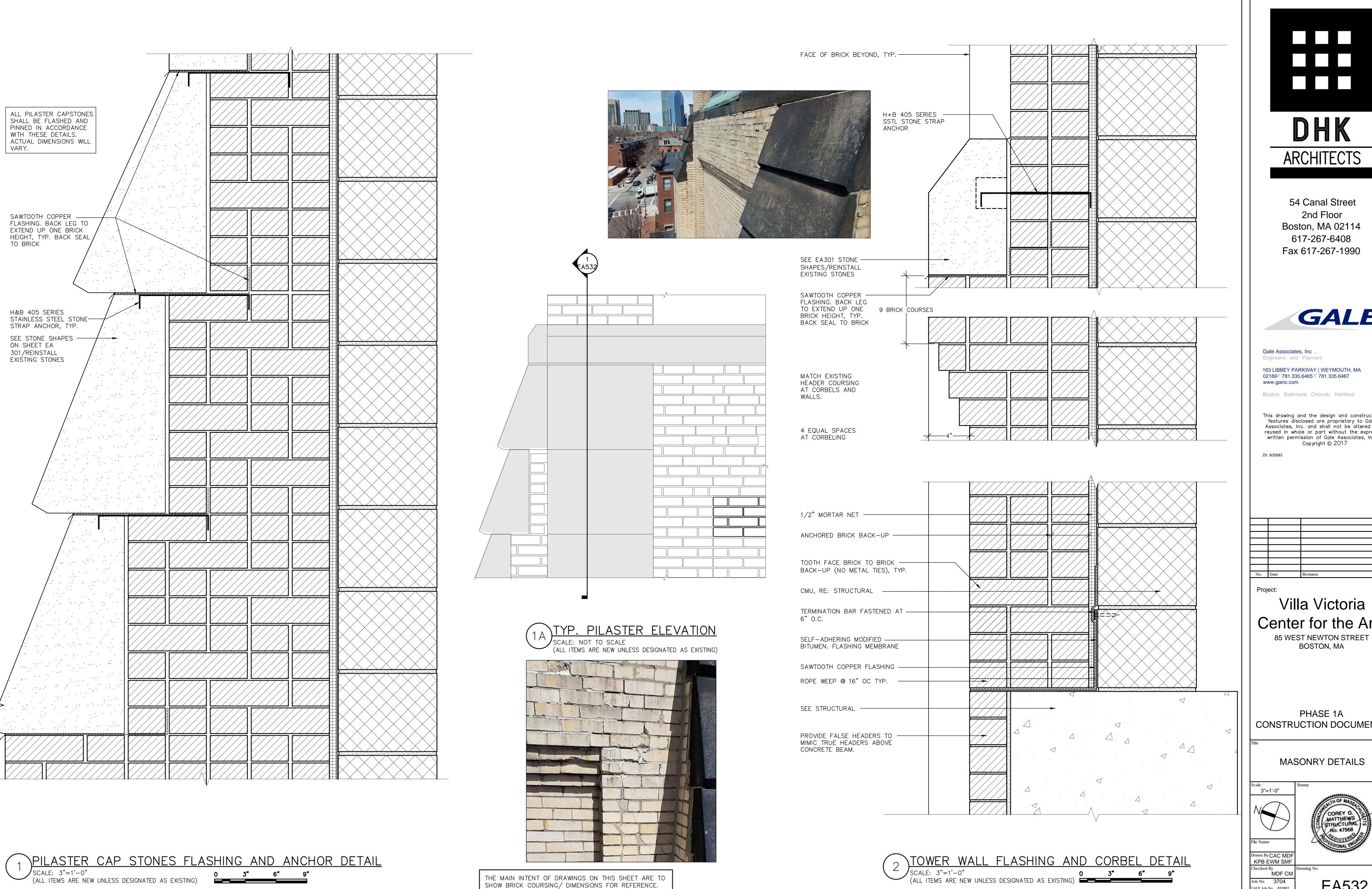
EA531

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TOWER WALL BRICK PATTERNS AND COURSING DETAILS

(ALL ITEMS ARE NEW UNLESS DESIGNATED AS EXISTING)





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GALE

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PHASE 1A CONSTRUCTION DOCUMENTS

MASONRY DETAILS

Job No. 3704 GALE Job No. 832681

9/22/2017

EA532

GENERAL NOTES (STAIRS):

- STRUCTURAL DRAWINGS SHALL BE USED IN CONJUNCTION WITH ARCHITECTURAL. MECHANICAL, ELECTRICAL DRAWINGS AND PROJECT SPECIFICATIONS. REFER TO ARCHITECTURAL DRAWINGS FOR DIMENSIONS AND/OR ELEVATIONS NOT SHOWN ON THE STRUCTURAL DRAWINGS.
- 2. THE INFORMATION SHOWN ON THE DRAWINGS HAS BEEN COMPILED FROM VARIOUS SOURCES AND MAY NOT REFLECT THE ACTUAL CONDITIONS AT THE TIME OF CONSTRUCTION. ALL DIMENSIONS AND CONDITIONS MUST BE FIELD VERIFIED AND ANY DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT AND ENGINEER BEFORE PROCEEDING WITH THE AFFECTED PORTION OF THE WORK.
- 3. SHOP DRAWINGS FOR REINFORCING STEEL (INCLUDING ALL ACCESSORIES), POUR SCHEDULES, CONCRETE MIX DESIGN, REINFORCED MASONRY AND STRUCTURAL STEEL SHALL BE SUBMITTED TO THE ENGINEER AND STAMPED ACCEPTANCE RECEIVED PRIOR TO FABRICATION. ERECTION SHALL PROCEED BASED ON ACCEPTED SHOP DRAWINGS ONLY.
- 4. GALE'S STAIR DESIGN IS LIMITED TO THE MAIN ENTRANCE STAIR CONDITIONS ONLY AND DOES NOT INCLUDE ENTRANCE, DOORWAY, THRESHOLD, INTERIOR SIGNAGE, OR BUILDING INTERIOR EVALUATION OF ACCESSIBILITY REQUIREMENTS.
- 5. NOTIFY THE ENGINEER OF ANY ARCHITECTURAL MODIFICATION OR DIMENSION CHANGES THAT MAY AFFECT THE STRUCTURAL DESIGN.
- 6. THE OWNER IS REQUIRED TO HIRE A CERTIFIED INDEPENDENT TESTING FIRM TO OBSERVE, REVIEW, TEST, AND REPORT ON SOILS, CONCRETE, REBAR, FORMWORK, ETC. AS OUTLINED BY THESE DESIGN DRAWNGS AND PROJECT SPECIFICATIONS. THE CONTRACTOR MUST CONTACT. COORDINATE, AND SCHEDULE THE TESTING FIRM THROUGH THE ENTIRE PROJECT DURATION.
- 7. THE CONTRACTOR IS REQUIRED TO INCLUDE ALL NECESSARY LABOR, MATERIALS, EQUIPMENT, APPARATUS, CURING MEANS AND METHODS, MOBILIZATION, PERMITTING, TEMPORARY PROTECTION, SHORING, BRACING, ETC. TO COMPLETE THE WORK.
- 8. THE CONTRACTOR IS RESPONSIBLE FOR FULLY CLEANING THE WORK AREA ON A DAILY BASIS, REMOVING AND PROPERLY DISPOSING OF ALL DEMOLISHED MATERIALS.
- 9. ANY DAMAGE TO THE PROPERTY, BUILDINGS, VEHICLES, ETC. AS A RESULT OF THE WORK, MUST BE REPAIRED OR REPLACED BY THE CONTRACTOR AT NO ADDITIONAL COST, TO THE SATISFACTION OF THE OWNER AND ENGINEER.
- 10. WORK HOURS MUST BE APPROVED BY AND COORDINATED WITH THE OWNER, PRIOR TO STARTING ANY WORK.
- 11. THE OWNER IS REQUIRED TO PROVIDE UNOBSTRUCTED ACCESS TO ALL WORK AREAS FOR THE CONTRACTOR.
- 12. ACCESS TO THE EXTERIOR THROUGH THE MAIN ENTRANCE DOORS WILL BE CLOSED OFF DURING CONSTRUCTION AND MUST REMAIN CLOSED UNTIL MATERIALS HAVE ACHIEVED ACCEPTABLE CURE. THE CONTRACTOR MUST COORDINATE THIS EFFORT WITH THE OWNER AND FIRE DEPARTMENT. THE CONTRACTOR MUST PROVIDE ALTERNATE SAFE AND ACCEPTABLE (FIRE DEPARTMENT APPROVED) EXITS UNTIL THE CONSTRUCTION IS COMPLETE.
- 13. GENERAL CONTRACTOR TO CONFORM TO ALL LOCAL AND STATE BUILDING CODE REQUIREMENTS.
- 14. NOTIFY THE ENGINEER OF ANY ARCHITECTURAL MODIFICATION OR DIMENSION CHANGES THAT MAY AFFECT THE STRUCTURAL DESIGN.
- 15. THE INFORMATION SHOWN ON THE DRAWINGS HAS BEEN COMPILED FROM VARIOUS SOURCES AND MAY NOT REFLECT THE ACTUAL CONDITIONS AT THE TIME OF CONSTRUCTION. CONTRACTOR MUST FIELD VERIFY PRIOR TO SUBMITTING BID.
- 16. THE CONTRACTOR IS RESPONSIBLE FOR CONTACTING "DIG SAFE" AND PERFORMING ALL NECESSARY INVESTIGATIONS TO LOCATE AND CLEARLY MARK ALL UTILITIES AND OTHER SUBTERRANEAN ITEMS PRIOR TO CONSTRUCTION. THE OWNER WILL ASSIST IN LOCATING EXISTING WATER LINES AND DRAINAGE SYSTEMS. CONTRACTOR TO SECURE WRITTEN APPROVAL FROM THE OWNER PRIOR TO INITIATING CONSTRUCTION.
- 5. ALL WORK SHALL CONFORM TO THE REQUIREMENTS OF THE LATEST EDITIONS OF THE FOLLOWING BUILDING CODES AND STANDARDS:
 - 9TH EDITION OF THE COMMONWEALTH OF MASSACHUSETTS STATE BUILDING
 - CODE (MSBC) ACI 318 — BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE.
 - ACI 530 BUILDING CODE REQUIREMENTS FOR MASONRY CONSTRUCTION. AISC SPECIFICATION FOR THE DESIGN, FABRICATION AND ERECTION OF
 - STRUCTURAL STEEL FOR BUILDINGS. • 521 CMR: ARCHITECTURAL ACCESS BOARD (SECTION 24 - RAMPS)

100 PSF

2. DESIGN LOADS BUILDING CATEGORY - III

STAIRS: LIVE LOAD

ROOF:

DEAD LOADS: STRUCTURE ACTUAL WEIGHT

LIVE LOADS: 45 PSF GROUND SNOW LOAD IMPORTANCE FACTOR, IS 1.0

EXPOSURE FACTOR, CE 1.0 THERMAL FACTOR, CT 1.2

DRIFT, SLIDING AND UNBALANCED LOADS IN ACCORDANCE WITH ASCE 7-10

40 PSF

FLOOR: DEAD LOADS: STRUCTURE

ACTUAL WEIGHT LIVE LOADS:

BELFRY FLOOR

SEISMIC CRITERIA: (NON-STRUCTURAL COMPONENTS)

COMPONENT IMPORTANCE FACTOR, IP = 1.0

RISK CATEGORY = III SEISMIC DESIGN CATEGORY = C

SITE CLASS E (UNKNOWN SOIL CONDITIONS)

SPECTRAL ACCELERATION (SHORT PERIOD), SDS = 0.357

COMPONENT AMPLIFICATION FACTOR, $A_P = 2.5$ COMPONENT RESPONSE MODIFICATION FACTOR, $R_p = 1.0$

COMPONENT OPERATING WEIGHT, W

WIND CRITERIA: (ASCE 7-10 - DIRECTIONAL PROCEDURE)

BASIC WIND SPEED, V = 139MPH (ULTIMATE)

EXPOSURE CATEGORY = B

RISK CATEGORY = III (OCCUPANCY GREATER THAN 300) EXPOSURE COEFFICIENT, K₇ (TABLE 27.3-1)

TOPOGRAPHIC FACTOR COEFFICIENT, $K_{7T} = 1.0$

DIRECTIONALITY FACTOR, $K_D = 0.95$

GUST FACTOR, G = 0.85

VELOCITY PRESSURE, $Q_Z = 0.00256 * K_Z * K_{ZT} * K_D * V_2$ (PSF)

DEMOLITION NOTES:

- 1. THE INFORMATION SHOWN ON THESE DRAWINGS HAS BEEN COMPILED FROM VARIOUS SOURCES, AND MAY NOT REFLECT THE ACTUAL CONDITIONS AT THE TIME OF CONSTRUCTION. ALL DIMENSIONS AND FOOTING LOCATIONS ARE BASED ON ENGINEERING ASSUMPTIONS.
- 2. SCOPE OF DEMOLITION WORK INCLUDES THE REMOVAL OF ALL MATERIALS ASSOCIATED WITH EXISTING STAIRS, INCLUDING EXISTING FOUNDATIONS, GRAVEL BACKFILL, BRICK MASONRY PLANTER, HANDRAILS ETC. AND AS SHOWN ON THE DEMOLITION PLANS. ALL EXISTING GRANITE STAIR TREADS SHALL BE SALVAGED AND RE-INSTALLED AT A LATER POINT DURING THE CONSTRUCTION.
- 3. CONTRACTOR IS TO FIELD-VERIFY AND CATALOGUE ALL STAIR DIMENSIONS AND TREAD ASSOCIATED CLEARANCES. EXISTING STONES WILL BE RE-SET AND ALIGNED IN LIKE-KIND ATOP OF NEW FOUNDATION BASE.
- 4. CONTRACTOR MAY REMOVE THE EXISTING STONE TREADS WITH CAUTION TO MAINTAIN INTEGRITY OF THE EXISTING MARTIAL FOR FURTHER USE.
- 5. THE CONTRACTOR SHALL USE CAUTION WHILE PERFORMING DEMOLITION WORK ADJACENT TO THE EXISTING BUILDING TO AVOID UNDERMINING EXISTING MASONRY FOUNDATIONS. DO NOT IMPOSE LATERAL SURCHARGE OR IMPACT FORCES TO EXISTING BUILDING COMPONENTS, SYSTEMS, OR FOUNDATIONS AT ANY TIME.
- CONTRACTOR IS TO INFORM ENGINEER IF EXISTING GRANITE STONES EXPOSE HEAVY SPALLING, CRACKING OR OTHER DETERIORATION WHICH CAN IMPEDE THE RE-USE OF MATERIAL.
- 7. ALL ITEMS NOTED TO BE REMOVED OR DEMOLISHED (NOT INDICATED TO BE REUSED) MUST BE PROPERLY DISPOSED OF IN ACCORDANCE WITH MASSACHUSETTS REGULATIONS.
- THE CONTRACTOR IS RESPONSIBLE FOR CONTACTING "DIG SAFE" AND PERFORMING ALL NECESSARY INVESTIGATIONS TO LOCATE AND CLEARLY MARK ALL UTILITIES AND OTHER SUBTERRANEAN ITEMS PRIOR TO CONSTRUCTION. THE OWNER WILL ASSIST IN LOCATING EXISTING WATER LINES AND DRAINAGE SYSTEMS. CONTRACTOR TO SECURE WRITTEN APPROVAL FROM THE OWNER PRIOR TO INITIATING CONSTRUCTION.
- 9. ALL EXISTING UTILITIES, ELECTRICAL CONDUITS, ETC. WHICH INTERFERE IN THE REPAIR WORK SHALL BE REMOVED, REPOSITIONED AND RE-CONNECTED AFTER COMPLETION OF WORK BY THE CONTRACTOR. AS THE WORK PROGRESSES IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO CONTRACT, COORDINATE, FIRE AND PAY FOR LICENSED ELECTRICIANS, PLUMBERS AND HVAC SUBCONTRACTORS TO COMPLETE THE TEMPORALLY REMOVAL AND RE-CONNECTION / RELOCATION OF ALL BUILDING SERVICES UNLESS OTHERWISE.
- 10. DEMOLITION TO COMPLY WITH GOVERNING EPA NOTIFICATION REGULATIONS BEFORE STARTING SELECTIVE DEMOLITION. COMPLY WITH HAULING AND DISPOSAL REGULATIONS OF AUTHORITIES HAVING JURISDICTION.
- 11. SURVEY THE CONDITION OF THE SITE TO DETERMINE WHETHER REMOVING ANY ELEMENT MIGHT RESULT IN UNDESIRABLE DAMAGE OF ANY PORTION OF ADJACENT FACILITIES DURING SELECTIVE DEMOLITION. NOTIFY GALE IF FOUND, PRIOR TO INITIATING THE WORK.
- 12. MAINTAIN EXISTING UTILITIES INDICATED TO REMAIN IN SERVICE AND PROTECT THEM AGAINST DAMAGE DURING SELECTIVE DEMOLITION OPERATIONS.
- 13. CONDUCT DEMOLITION OPERATIONS AND REMOVE DEBRIS TO ENSURE MINIMUM INTERFERENCE WITH ROADS, PARKING LOTS, STREETS, WALKS, AND OTHER ADJACENT OCCUPIED AND USED FACILITIES.
- 14. CONDUCT DEMOLITION OPERATIONS TO PREVENT INJURY TO PEOPLE AND DAMAGE TO ADJACENT BUILDINGS, FACILITIES, AND SITE IMPROVEMENTS TO REMAIN. ENSURE SAFE PASSAGE OF PEOPLE AROUND SELECTIVE DEMOLITION AREA.
- 15. USE WATER MIST AND OTHER SUITABLE METHODS TO LIMIT THE SPREAD OF DUST AND DIRT. COMPLY WITH GOVERNING ENVIRONMENTAL PROTECTION REGULATIONS.
- 16. REMOVE AND TRANSPORT DEBRIS IN A MANNER THAT WILL PREVENT SPILLAGE ON ADJACENT SURFACES AND AREAS.
- 17. CLEAN ADJACENT STRUCTURES AND IMPROVEMENTS OF DUST, DIRT, AND DEBRIS CAUSED BY SELECTIVE DEMOLITION OPERATIONS. RETURN ADJACENT AREAS TO CONDITION EXISTING BEFORE START OF SELECTIVE DEMOLITION.
- 18. DEMOLISH AND REMOVE EXISTING CONSTRUCTION ONLY TO THE EXTENT REQUIRED BY NEW CONSTRUCTION AND AS INDICATED. CONTRACTOR TO BE RESPONSIBLE FOR ANY CUTTING AND PATCHING THAT IS REQUIRED.
- 19. PROMPTLY DISPOSE OF DEMOLISHED MATERIALS. DO NOT ALLOW DEMOLISHED MATERIALS TO ACCUMULATE ON-SITE.
- 20. DO NOT BURN DEMOLISHED MATERIALS.
- 21. TRANSPORT DEMOLISHED MATERIALS OFF OWNER'S PROPERTY AND LEGALLY DISPOSE OF THEM, IF NOT DESIGNATED TO BE SALVAGED BY THE OWNER OR REUSED.
- 22. IN AREAS WHERE CONCRETE IS TO BE REMOVED, THE EDGE OF ANY CONCRETE TO REMAIN MUST BE A CLEAN SAW-CUT EDGE OR AS SPECIFIED ON THE CONTRACT DRAWINGS.

CONCRETE NOTES:

- 1. CONCRETE WORK SHALL CONFORM TO THE LATEST AMERICAN CONCRETE INSTITUTE CODE FOR "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE".
- 2. SLAB CONCRETE SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 4000 PSI AT 28 DAYS WITH A SLUMP OF NO MORE THAN 2" TO 4" AND AIR ENTRAINMENT OF 4 1/2 - 71/2%. THE USE OF CALCIUM CHLORIDE IS NOT PERMITTED. PROVIDE PROPER CONCRETE PROTECTION OR HEAT IN COLD WEATHER AND MAINTAIN PROPER CURING PROCEDURES IN ACCORDANCE WITH ALL CURRENT A.C.I. STANDARDS. SLUMP MEASUREMENT CHANGES MAY BE ALLOWED IF MIX INCLUDES MID-RANGE, HIGH-RANGE OR SUPERPLASTICIZER ADMIX.
- 3. STEEL REINFORCEMENT SHALL CONFORM TO ASTM 615. GRADE 60. DEFORMED BARS.
- 4. WHERE CONTINUOUS BARS ARE CALLED FOR THEY SHALL BE RUN CONTINUOUSLY AROUND CORNERS AND LAPPED AT NECESSARY SPLICES OR HOOKED AT DISCONTINUOUS ENDS. LAPS SHALL BE 40 BAR DIAMETERS (MINIMUM), UNLESS OTHERWISE SHOWN. USE 2'-0"x 2'-0" CORNER BARS AT EACH CORNER FOR EACH CONTINUOUS HORIZONTAL BAR.
- 5. ALL REINFORCING SHALL BE ELECTRICALLY GROUNDED IN ACCORDANCE WITH THE NATIONAL ELECTRIC CODE, AS AMENDED BY THE MA STATE BUILDING CODE. COORDINATE WITH ELECTRICAL ENGINEER.
- 6. ALL REINFORCING BARS SHALL BE COLD BENT IN ACCORDANCE TO THE PROPER RADII ESTABLISHED BY THE AMERICAN CONCRETE INSTITUTE. UNDER NO CONDITIONS SHALL HEAT BE APPLIED TO THE BARS TO OBTAIN BENDS.
- 7. FORMS SHALL BE OILED PRIOR TO THEIR ERECTION. REINFORCING BARS WHICH ARE COATED WITH FORM OIL OR ANY OTHER BOND BREAKING MATERIAL WILL BE REJECTED AND WILL REQUIRE REPLACEMENT AT NO ADDITIONAL COST TO THE OWNER.
- 8. CONCRETE SHALL NOT CONTAIN SLAG OR SILICA FUME. IF USING FLY-ASH, THE MAXIMUM QUANTITY PER WEIGHT OF CEMENT SHALL BE 20%. SUBMIT FLY-ASH CERTIFICATION FOR APPROVAL WITH MIX DESIGNS.
- 9. SUBMIT COMPLETE REINFORCING STEEL SHOP DRAWINGS ALONG WITH COMPLETE CONCRETE MIX DESIGN (INCLUDING ALL ADDITIVES AND THEIR CONTENT) TO THE ENGINEER FOR REVIEW AND APPROVAL PRIOR TO FABRICATING STEEL.
- 10. ADDITION OF WATER TO CONCRETE MIXES AT THE SITE IS NOT ALLOWED. SUCH CONCRETE SHALL BE IMMEDIATELY REJECTED. MID-RANGE, OR HIGH RANGE, OR SUPERPLASTICIZERS ARE ALLOWED IN THE MIX TO ASSIST IN WORKABILITY AND PUMPING OPERATIONS, AND MIX-DESIGN MAKE-UP WATER MAY BE POST SUPPLEMENTED AT THE SITE. TESTING FIRM MUST MEASURE AND REPORT.
- 11. CONCRETE CEMENT SHALL BE TYPE I OR II PORTLAND CEMENT. MAXIMUM AGGREGATE SIZE IN CONCRETE SHALL BE 1".
- 12. ALL CONCRETE SHALL BE READI-MIXED AT PLANT COMPLYING WITH ASTM C94 AND ASTM C1116. SITE MIXING IS NOT ALLOWED.
- 13. ALL CONCRETE SHALL BE REINFORCED AS SHOWN ON THE DRAWINGS. PROVIDE SUFFICIENT CHAIR OR SUPPORT BARS AS NECESSARY TO PROPERLY POSITION REINFORCING STEEL. PULL UP OF BARS OR MESH, OR UNSUPPORTED BARS OR MESH WILL NOT BE ALLOWED. "WET STICKING" OF BARS WILL NOT BE ALLOWED.
- 14. ALL CONCRETE SHALL BE POURED TO THE SPECIFIED THICKNESS AND REINFORCED AS SHOWN ON THE DRAWINGS. WELDED WIRE MESH REINFORCEMENT SHALL CONFORM TO A.S.T.M. A185, AND SHALL LAP 6" MINIMUM OR ONE SPACE, WHICHEVER IS LARGER. AND SHALL BE WIRED TOGETHER. IT IS REQUIRED THAT CHAIR BARS BE USED TO MAINTAIN PROPER LOCATION OF WELDED WIRE MESH.
- 15. CONCRETE USED FOR ALL (SLABS AND BEAMS) SHALL BE TESTED BY AN INDEPENDENT ACI CERTIFIED TESTING LAB, HIRED, AND PAID FOR BY THE OWNER. THE CONTRACTOR IS REQUIRED TO CONTACT AND COORDINATE THE TESTING LAB SERVICES. THE FOLLOWING MINIMUM TESTING SHALL BE PERFORMED, AND FIELD/LAB-RESULT REPORTS SUBMITTED TO
- THE STRUCTURAL ENGINEER FOR APPROVAL:
- AIR ENTRAINMENT AT PLACEMENT ASTM C-231-97 ASTM C-143
- SLUMP -ASTM C-39COMPRESSIVE STRENGTH —

CONCRETE CYLINDER SAMPLES SHALL BE OBTAINED FROM EACH CONCRETE DELIVERY TRUCK FOR COMPRESSIVE STRENGTH TESTING. FIVE (5) CYLINDERS SHALL BE MADE FROM EACH SAMPLE. EACH CYLINDER SHALL BE STANDARD 6" DIAMETER BY 12" TALL. ONE (1) CYLINDER WILL BE TESTED AT 7-DAY CURE, AND THREE (3) CYLINDERS WILL BE TESTED AT 28-DAY CURE TO DETERMINE COMPRESSIVE STRENGTH OF THE CONCRETE IN ACCORDANCE WITH ASTM C-39. AIR ENTRAINMENT AND SLUMP WILL BE TESTED AT EACH SAMPLE AS WELL. RETAIN THE FIFTH CYLINDER SAMPLE FOR POTENTIAL 56 DAY COMPRESSIVE TESTING AND/OR PETROGRAPHIC EXAMINATION. TEST RESULTS WHICH ARE DETERMINED BY GALE TO BE DEFICIENT OR QUESTIONABLE WILL REQUIRE THAT THE CONTRACTOR PAY FOR ADDITIONAL TESTING AND CORING OF THE IN-PLACE CONCRETE INCLUDING PETROGRAPHIC EXAMINATION WITH REPORT AS DIRECTED BY GALE. CONCRETE DETERMINED BY GALE TO REMAIN DEFICIENT AFTER FINAL TESTING SHALL BE ENTIRELY REMOVED AND REPLACED AT NO ADDITIONAL COST.

28. CHAIR BARS FOR SECURE PLACEMENT AND POSITIONING OF REINFORCING STEEL IS TO BE PROVIDED. CHAIR BAR OR SIMILAR APPROVED MANUFACTURED DEVICES INTENDED FOR USE MUST BE SUBMITTED TO GALE AND APPROVED IN WRITING PRIOR TO ORDERING MATERIALS. REINFORCING SUPPORTS SHALL BE OF PROPER HEIGHT, LENGTH, SPACING, SIZE AND MATERIAL TYPE; AND SUBMITTAL SHALL INCLUDE THIS DATA WITH CURRENT MANUFACTURER DATA SHEETS. IN NO CASE SHALL BRICK, WOOD OR OTHER NON-CONFORMING REINFORCING STEEL SUPPORTS BE USED.

METAL ROOF DECK NOTES:

- 1. STEEL ROOF DECK SHALL BE 1½" DEPTH 20 GAUGE, TYPE B, AS MANUFACTURED BY VULCRAFT, OR AN ENGINEER APPROVED EQUIVALENT PRODUCT OF ANY MEMBER OF THE STEEL DECK INSTITUTE (SDI) AND SHALL CONFORM TO ASTM DESIGNATION A446 GRADE A (FOR GALVANIZED DECK).
- 2. MINIMUM STRUCTURAL PROPERTIES BASE ON MINIMUM YIELD STRENGTH OF 33,000 PSI WITH A WORKING STRESS NOT TO EXCEED 20,000 PSI.
- 3. THE DEFLECTION OF THE DECK UNDER DESIGN LIVE LOAD SHALL NOT EXCEED L/240 OF THE SPAN.
- 4. ROOF DECK SHALL BE CAPABLE OF SUPPORTING THE DESIGN LOADS SHOWN ON THE DRAWINGS AND AS REQUIRED BY REFERENCED DESIGN CODES.
- 5. ALL DECK SHALL BE HOT-DIP GALVANIZED. GALVANIZING SHALL CONFORM TO THE REQUIREMENTS OF ASTM A525 COATING CLASS G60.
- 6. DECK SHALL BE FASTENED TO THE STRUCTURAL SUPPORTS MUST BE AS PER MANUFACTURER'S RECOMMENDATIONS, SDI REQUIREMENTS, OR AS DIRECTED BY THE ENGINEER. FASTENING PATTERN FOR END LAPS MUST BE A 36/4. ALL SCREWS SHALL PENETRATE 34" MINIMUM INTO BEAM FLANGE OR ANGLE LEG WHEN SEAL IS FULLY SEATED ON DECK.
- 7. DECK SHEETS SHALL EXTEND CONTINUOUSLY OVER MINIMUM OF TWO (2) SPANS.
- 8. DECK DESIGN CAPACITIES, DETAILS, INSTALLATION REQUIREMENTS, FASTENER PATTERNS, REQUIRED LAPS, PLANS, ETC. SHALL BE SUBMITTED TO THE ENGINEER FOR REVIEW AND APPROVAL PRIOR TO FABRICATION AND INSTALLATION.
- 9. END LAPS OF METAL DECK SHEETS SHALL BE MINIMUM OF 2" AND SHALL OCCUR OVER SUPPORTS WITH SPECIFIED FASTENING.
- 10. SIDELAP FASTENING SHALL BE SELF-TAPPING MACHINE SCREWS SPACED AT 24" O.C.

CONCRETE MASONRY NOTES:

- 1. ALL REINFORCED CONCRETE MASONRY SHALL CONFORM TO THE LATEST EDITIONS OF THE NCMA "SPECIFICATION FOR THE DESIGN AND CONSTRUCTION OF LOAD-BEARING CONCRETE MASONRY," AND ACI 530 "BUILDING CODE REQUIREMENTS FOR MASONRY CONSTRUCTION.
- 2. CONCRETE MASONRY UNITS SHALL CONFORM TO ASTM C90, GRADE N, TYPE I NORMAL WEIGHT BLOCK AND HAVE A MINIMUM COMPRESSIVE STRENGTH OF 4800 PSI ON THE NET AREA. BRICK MASONRY SHALL CONFORM TO ASTM C62, GRADE SW AND HAVE A MINUMUM COMPRESSIVE STRENGTH OF 2500 PSI ON THE NET AREA.
- 3. MORTAR SHALL CONFORM TO ASTM C270, TYPE S MORTAR FOR THE REINFORCED CONCRETE MASONRY AND TYPE N FOR THE BRICK MASONRY.
- 4. GROUT TO FILL CELLS AND BOND BEAMS SHALL CONFORM TO ASTM C476, FINE TYPE, AND SHALL HAVE A MINIMUM 28 DAY COMPRESSIVE STRENGTH OF 3000 PSI:
- a. ALL GROUTING PROCEDURES SHALL CONFORM TO NCMA "TEK SERIES #23A" 9-02B AND 9-04A.
- b. GROUT SLUMP SHALL BE BETWEEN 8 AND 11 INCHES. SUBMIT MIX DESIGN TO ENGINEER FOR APPROVAL.
- 5. ALL REINFORCING BARS SHALL CONFORM TO ASTM A615, GRADE 60, DEFORMED.
- PROVIDE HORIZONTAL AND VERTICAL REINFORCING AS NOTED ON THE DRAWINGS AND FILL ALL CELLS WITH REINFORCING WITH GROUT. REFER TO "MINIMUM WALL REINFORCING SCHEDULE," OR SECTIONS, FOR REQUIRED WALL REINFORCEMENT. HORIZONTAL REINFORCEMENT SHALL BE CONTINUOUS THROUGH CORNERS, INTERSECTIONS AND PILASTERS, AND SHALL CONSIST OF TRUSS OR LADDER NO. 9 WIRE SPACED NOT MORE THAN 4'-0" ON CENTER.
- GROUTING SHALL BE LIMITED TO A MAXIMUM WALL HEIGHT OF 64 INCHES PER LIFT, AND MUST BE VERIFIED FOR FULL HEIGHT SOLID PLACEMENT VIA CODE REQUIRED CLEAN OUTS.
- 8. ALL CMU DESIGN AND CONSTRUCTION SHALL CONFORM TO THE REQUIREMENTS OF CHAPTERS 1 THROUGH 8 OF ASCE/ACI 530. PLACE CMU IN RUNNING BOND.
- 9. SET REINFORCING BARS AS INDICATED. PROVIDE VERTICAL AND HORIZONTAL LAP SPLICES. 40 BAR DIAMETERS MINIMUM OR OTHERWISE INDICATED. TIE BARS AND CENTER BARS IN CELL. INSPECT ALL CELLS FOR ALIGNMENT AND DEBRIS IN CELL PRIOR TO GROUTING.
- 10. CONTRACTOR TO NOTE THAT FLASHING, WEEPS, END DAMS, SEALANTS AND DRIP-EDGES ARE REQUIRED AT LOCATIONS SHOWN ON THE DRAWINGS AND SPECIFICATIONS FOR REQUIRED WORK AND MATERIALS.

STRUCTURAL STEEL:

- 1. ALL STRUCTURAL STEEL MATERIALS, WORKMANSHIP, AND DETAILS, SHALL CONFORM TO THE LATEST EDITION OF THE ANSI/AISC 360 "SPECIFICATION FOR THE STRUCTURAL STEEL BUILDINGS" AND AISC "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES". STRUCTURAL STEEL SHALL CONFORM TO THE FOLLOWING SPECIFICATIONS
 - STRUCTURAL STEEL SHAPES W SHAPES GRADE 50
 - STRUCTURAL STEEL PLATES ASTM A36 MIN.
 - STEEL ANGLES AND CHANNELS ASTM A36 MIN.
- 2. ALL SHOP CONNECTIONS SHALL BE WELDED TO CONFORM TO "STRUCTURAL WELDING CODE", AWS D1.1, LATEST EDITION OF THE AMERICAN WELDING SOCIETY, E70 SERIES. SHOP CONNECTIONS MAY BE HIGH STRENGTH BOLTED TO CONFORM TO ASTM A325.
- 3. ALL BOLTED CONNECTIONS SHALL BE HIGH STRENGTH BOLTED TO CONFORM TO ASTM A325-N, UNO. WHERE WELDING IS SPECIFIED, WELDING PER NOTE ABOVE SHALL APPLY.
- 4. ANCHOR BOLTS MUST BE CAST INTO CONCRETE AND SET/ TIED IN PLACE VIA TEMPLATE. WET-STICKING OF ANCHOR BOLTS INTO CONCRETE IS NOT ALLOWED.
- 5. ANCHOR RODS SHALL BE GRADE 60, UNO. AND SHALL BE HOT-DIP GALVANIZED. EMBEDMENT INTO CONCRETE SHALL BE 1'-4" MINIMUM WITH A 3" LONG 90° HOOK OR HEX-HEADED BOLT HEAD, UNO. SET ANCHOR RODS WITH TEMPLATE TO ENSURE PROPER POSITIONING.
- 6. ALL SIMPLY SUPPORTED BEAM CONNECTIONS SHALL CONFORM TO THE TYPICAL DETAILS AND SHALL BE DESIGNED BY THE STEEL FABRICATOR AND SUBMITTED ON STEEL SHOP DRAWINGS. INCLUDE ENGINEER'S CERTIFIED CALCULATIONS.
- 7. ALL WELDING SHALL BE PERFORMED BY AWS CERTIFIED WELDERS. CERTIFICATIONS MUST BE CURRENT WHEN WELDING IS PERFORMED.
- 8. PROVIDE A MINIMUM OF 1/4" FILLET WELDS (ALL AROUND) AT WELDED CONNECTIONS,
- 9. ALL STEEL SHALL BE NEW STEEL CONFORMING TO THE AISC SPECIFICATIONS FOR DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS. 10. ALL WELDS SHALL DEVELOP THE FULL STRENGTH OF THE MATERIAL BEING WELDED. USE
- EXX 70 ELECTRODES. 11. NO PERMANENT CONNECTIONS SHOULD BE MADE UP UNTIL THE STRUCTURE HAS BEEN PROPERLY ALIGNED. PROVIDE TEMPORARY BRACING AS REQUIRED.
- 12. SUBMIT THREE (3) COPIES OF SHOP DRAWINGS TO THE ENGINEER SHOWING SETTING PLANS, ERECTION PLANS, ALL DETAILS AND SIZES OF MEMBERS INCLUDING CONNECTIONS AND ALL ENGINEERING CALCULATIONS. STEEL FABRICATOR IS RESPONSIBLE FOR FINAL CONNECTION DETAILS AND DESIGN IN ACCORDANCE WITH THE MINIMUM REQUIREMENTS OF THE LATEST EDITION OF THE AISC DETAILING MANUAL.
- 13. CONNECTION BOLTS TO BE 3/4" DIAMETER HIGH STRENGTH, ASTM A325. PROVIDE A MINIMUM OF THREE (3) BOLTS PER CONNECTION.



54 Canal Street 2nd Floor Boston, MA 02114 617-267-6408 Fax 617-267-1990



Gale Associates, Inc.

Engineers and Planners 163 LIBBEY PARKWAY | WEYMOUTH, MA 02189P 781.335.6465 F 781.335.6467

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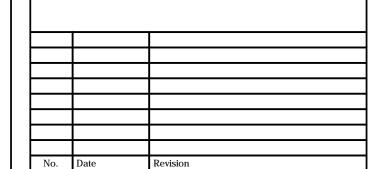
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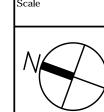
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CONSTRUCTION DOCUMENTS

TECHNICAL NOTES



KPB EWM SMF

MDF CM CGM Job No. 3704 GALE Job No. 832681

9/22/2017

GRANITE STONE NOTES (STAIRS):

- 1. AFTER REMOVAL, GRANITE SHALL BE STORED ON WOOD SKIDS OR PALLETS, COVERED WITH NON-STAINING, WATERPROOF MEMBRANE AND PROTECTED FROM THE WEATHER. SKIDS SHALL BE PLACED AND STACKED IN SUCH A MANNER AS TO EVENLY DISTRIBUTE THE WEIGHT OF THE GRANITE MATERIALS AND TO PREVENT DAMAGE TO GRANITE PIECES. GRANITE MATERIALS SHALL BE STORED IN SUCH A MANNER AS TO ALLOW AIR TO CIRCULATE AROUND THE MATERIAL. GRANITE SHALL NOT BE PERMITTED TO BE IN DIRECT CONTACT WITH THE GROUND ANY TIME DURING STORAGE.
- 2. GRANITE SHALL BE CAREFULLY HANDLED TO PREVENT CHIPPING, BREAKAGE, SOILING OR OTHER DAMAGE. PINCH OR WRECKING BARS SHALL NOT BE USED WITHOUT PROTECTING EDGES OF GRANITE WITH WOOD OR OTHER RIGID MATERIALS. GRANITE UNITS SHALL BE LIFTED WITH WIDE-BELT TYPE SLINGS WHEREVER POSSIBLE; WIRE ROPE OR ROPES CONTAINING TAR OR OTHER SUBSTANCES WHICH MIGHT CAUSE STAINING OR DAMAGE TO GRANITE FINISH WILL NOT BE PERMITTED.
- 3. MATERIALS ASSOCIATED WITH THE GRANITE TREAD PLACEMENT SHALL BE:

a. <u>STEEL DOWELS:</u>

EACH STONE BEARING END SHALL BE PROVIDED WITH A MINIMUM OF ONE (1) #5 DOWEL EMBEDDED FOUR (4) INCHES WITHIN THE GRANITE STONE AND A MINIMUM OF SIX (6) INCHES WITHIN THE CONCRETE FOUNDATION OR CMU WALL. REFER TO DESIGN DETAILS FOR FURTHER SPECIFICATIONS.

b. <u>GROUT:</u>

GRANITE BLOCK SETTING BEDS SHALL BE A NON-SHRINK, HIGH PERFORMANCE, CEMENTITIOUS GROUT CONFORMING TO ASTM C 1107 (GRADE C) SUCH AS SIKAGROUT 212 AS MANUFACTURED BY SIKA, OR APPROVED EQUAL.

c. <u>MORTAR:</u>

MORTAR AT ALL GRANITE BLOCK JOINTS SHALL BE A NON-SHRINK, HIGH PERFORMANCE, CEMENTITIOUS GROUT CONFORMING TO ASTM C 1107 (GRADE C) SUCH AS SIKAGROUT 212 AS MANUFACTURED BY SIKA, OR APPROVED EQUAL

d. <u>SHIMS:</u>

SHIMS FOR ACHIEVING PROPER LEVELING OF GRANITE BLOCKS SHALL BE HIGH-DENSITY POLYETHYLENE SHIMS SUCH AS SHIM-PAKS, BY KOROLATH OF NEW ENGLAND, OR APPROVED EQUAL.

- 4. ALL SETTING SHALL BE DONE BY COMPETENT GRANITE SETTERS UNDER ADEQUATE SUPERVISION AND IN ACCORDANCE WITH THE APPROVED SHOP DRAWINGS.
- 5. PLACE GRANITE TREADS AND BLOCKS ON CONCRETE PIERS WITH 2-INCH MAXIMUM DEPTH GROUTED SETTING BED.
- 6. INSTALL BOTH MORTAR JOINTS AND SEALANT JOINTS AT GRANITE JOINTS. REFER TO CONTRACT DRAWINGS FOR FURTHER DETAILS.
- 7. BEFORE SETTING, GRANITE SHALL BE CLEAN AND FREE OF DIRT AND FOREIGN MATTER ON ALL SIDES. GRANITE SHALL BE DRY BEFORE SETTING. GRANITE SHALL BE SET ON BED OF GROUT AS 14. ALL REINFORCING BARS SHALL BE COLD BENT IN ACCORDANCE TO THE PROPER RADII SPECIFIED.
- 8. GRANITE SHALL BE SET TRUE TO THE REQUIRED LINES AND GRADES. JOINTS SHALL BE UNIFORM IN THICKNESS. DIRECT BEARING CONTACT BETWEEN GRANITE PIECES SHALL BE PROHIBITED.
- 9. EXPOSED SURFACES SHALL BE KEPT FREE FROM MORTAR AT ALL TIMES. ANY MORTAR SMEARS SHALL BE IMMEDIATELY REMOVED WITH A CLEAN SPONGE AND CLEAN WATER BEFORE LATEX-MODIFIED MORTAR CAN SET
- 10. HOLES, SLOTS, AND OTHER SINKAGES FOR ANCHORS AND DOWELS SHALL BE COMPLETELY FILLED WITH MORTAR DURING SETTING OF GRANITE.

MASONRY NOTES (STAIRS):

- 1. ALL REINFORCED CONCRETE MASONRY SHALL CONFORM TO THE LATEST EDITIONS OF THE NCMA "SPECIFICATION FOR THE DESIGN AND CONSTRUCTION OF LOAD-BEARING CONCRETE MASONRY," AND ACI 530 "BUILDING CODE REQUIREMENTS FOR MASONRY CONSTRUCTION.
- 2. MASONRY UNITS SHALL CONFORM TO ASTM C90, GRADE N, TYPE I NORMAL WEIGHT BLOCK AND HAVE A MINIMUM COMPRESSIVE STRENGTH OF 3000 PSI ON THE NET AREA.
- 3. MORTAR SHALL CONFORM TO ASTM C270, TYPE S MORTAR FOR THE REINFORCED CONCRETE MASONRY AND TYPE N FOR THE BRICK MASONRY.
- SHALL HAVE A MINIMUM 28 DAY COMPRESSIVE STRENGTH OF 3000 PSI, WITH THE FOLLOWING MIX DESIGN:
- a. GROUT PROPORTIONS:

3/8" PEASTONE	1650 LB.
SAND	1200 LB.
TYPE II PORTLAND CEMENT	6 SACKS
WATER	52 GALLONS
"WRDA" BY W.R. GRACE CO. (OR APPROVED EQUAL)	52 OZ.

- b. ALL GROUTING PROCEDURES SHALL CONFORM TO NCMA "TEK SERIES #23A" 9-02B
- c. GROUT SLUMP SHALL BE BETWEEN 8 AND 11 INCHES. SUBMIT MIX DESIGN TO ENGINEER FOR APPROVAL.
- 5. ALL REINFORCING BARS SHALL CONFORM TO ASTM A615, GRADE 60, DEFORMED.
- 6. PROVIDE HORIZONTAL AND VERTICAL REINFORCING AS NOTED ON THE DRAWINGS AND FILL ALL CELLS WITH REINFORCING WITH GROUT. REFER TO "MINIMUM WALL REINFORCING SCHEDULE," OR SECTIONS, FOR REQUIRED WALL REINFORCEMENT. HORIZONTAL REINFORCEMENT SHALL BE CONTINUOUS THROUGH CORNERS, INTERSECTIONS AND PILASTERS, AND SHALL CONSIST OF TRUSS OR LADDER NO. 9 WIRE SPACED NOT MORE THAN 1'-4" ON CENTER.
- 7. GROUTING SHALL BE LIMITED TO A MAXIMUM WALL HEIGHT OF 64 INCHES PER LIFT, AND MUST BE VERIFIED FOR FULL HEIGHT SOLID PLACEMENT VIA CODE REQUIRED CLEAN OUTS.
- 8. ALL CMU DESIGN AND CONSTRUCTION SHALL CONFORM TO THE REQUIREMENTS OF CHAPTERS 1 THROUGH 8 OF ASCE/ACI 530. PLACE CMU IN RUNNING BOND.
- 9. SET REINFORCING BARS AS INDICATED, PROVIDE VERTICAL AND HORIZONTAL LAP SPLICES. 40 BAR DIAMETERS MINIMUM. TIE BARS AND CENTER BARS IN CELL. INSPECT ALL CELLS FOR ALIGNMENT AND DEBRIS IN CELL PRIOR TO GROUTING.

CONCRETE AND FOUNDATION NOTES (STAIRS):

- CONCRETE SHALL BEAR LEVEL ON SPECIFIED VAPOR BARRIER AND CRUSHED STONE ATOP SPECIFIED FILTER FABRIC AND STRUCTURAL FILL OVER UNDISTURBED ACCEPTABLE SOIL (PROOF-ROLLED WITH AT LEAST SIX PASSES EACH WAY WITH A LARGE PLATE COMPACTOR) OR STRUCTURAL COMPACTED FILL HAVING AN ALLOWABLE BEARING CAPACITY OF 3500 POUNDS PER SQUARE FOOT (MINIMUM).
- 2. IF BEARING MATERIALS WITH A LOWER BEARING CAPACITY THAN 3500 POUNDS PER SQUARE FOOT ARE ENCOUNTERED (AS DETERMINED BY THE GEOTECHNICAL ENGINEER), AT THE SPECIFIED FLEVATIONS. THE UNSUITABLE MATERIAL SHALL BE REMOVED AND REPLACED WITH SUITABLE MATERIAL TO BE APPROVED BY THE STRUCTURAL AND GEOTECHNICAL ENGINEER.
- 3. THE ENGINEER ASSUMES NO RESPONSIBILITY FOR THE VALIDITY OF THE SUBSURFACE CONDITIONS.
- 4. NO CONCRETE SHALL BE PLACED IN WATER OR ON FROZEN GROUND.
- 5. ALL CONCRETE SHALL BE PROTECTED AGAINST FROST UNTIL PROJECT IS COMPLETED.
- 6. BACKFILL SHALL BE COMPACTED IN 8" THICK MAXIMUM LOOSE LIFTS AND COMPACTED TO 95% MODIFIED PROCTOR (CLASS 1), ASTM D1557.
- 7. BACKFILL SIMULTANEOUSLY ALONG EACH SIDE WITH SPECIFIED COMPACTED FILL.
- CONCRETE WORK SHALL CONFORM TO THE LATEST AMERICAN CONCRETE INSTITUTE CODE FOR "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE".
- 9. FOUNDATION AND SLAB CONCRETE SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 4500 PSI AT 28 DAYS WITH A SLUMP OF NO MORE THAN 4" AND AIR ENTRAINMENT OF 4½-7½%. THE USE OF CALCIUM CHLORIDE IS NOT PERMITTED. PROVIDE PROPER CONCRETE PROTECTION OR HEAT IN COLD WEATHER AND MAINTAIN PROPER CURING PROCEDURES IN ACCORDANCE WITH ALL CURRENT A.C.I. STANDARDS. SLUMP MEASUREMENT CHANGES MAY BE ALLOWED IF MIX INCLUDES MID-RANGE, HIGH-RANGE OR SUPERPLASTICIZER ADMIX.
- 10. STEEL REINFORCEMENT SHALL CONFORM TO ASTM 615, GRADE 60, DEFORMED BARS.
- 11. WHERE CONTINUOUS BARS ARE CALLED FOR THEY SHALL BE RUN CONTINUOUSLY AROUND CORNERS AND LAPPED AT NECESSARY SPLICES OR HOOKED AT DISCONTINUOUS ENDS. LAPS SHALL BE 40 BAR DIAMETERS (MINIMUM), UNLESS OTHERWISE SHOWN. USE 2'-0"X 2'-0" CORNER BARS AT EACH CORNER FOR EACH CONTINUOUS HORIZONTAL BAR.
- 12. NOTIFY ENGINEER FOR INSPECTION OF COMPLETED INSTALLATION OF REINFORCEMENT AT LEAST 2 WORK DAYS PRIOR TO SCHEDULED PLACEMENT OF CONCRETE.
- 13. PLACEMENT OF CONCRETE POURS SHOULD HAVE A VERTICAL 2"X4" KEY WITH CONTINUOUS REINFORCING (40 BAR DIAMETER MINIMUM) THRU THE CONSTRUCTION JOINT. CONSTRUCTION JOINTS SHALL BE PLACED NO CLOSER THAN 10 FT. FROM ANY CORNER.
- ESTABLISHED BY THE AMERICAN CONCRETE INSTITUTE. UNDER NO CONDITIONS SHALL HEAT BE APPLIED TO THE BARS TO OBTAIN BENDS.
- 15. FORMS SHALL BE OILED PRIOR TO THEIR ERECTION. REINFORCING BARS WHICH ARE COATED WITH FORM OIL OR ANY OTHER BOND BREAKING MATERIAL WILL BE REJECTED AND WILL REQUIRE REPLACEMENT AT NO ADDITIONAL COST TO THE OWNER.
- 16. CONCRETE SHALL NOT CONTAIN SLAG OR SILICA FUME. IF USING FLY-ASH, THE MAXIMUM QUANTITY PER WEIGHT OF CEMENT SHALL BE 20%. SUBMIT FLY-ASH CERTIFICATION FOR APPROVAL WITH MIX DESIGNS.
- 17. SUBMIT COMPLETE REINFORCING STEEL SHOP DRAWINGS ALONG WITH COMPLETE CONCRETE MIX DESIGN (INCLUDING ALL ADDITIVES AND THEIR CONTENT) TO THE ENGINEER FOR REVIEW AND APPROVAL PRIOR TO FABRICATING STEEL.
- 18. ADDITION OF WATER TO CONCRETE MIXES AT THE SITE IS NOT ALLOWED. SUCH CONCRETE SHALL BE IMMEDIATELY REJECTED. MID-RANGE, OR HIGH RANGE, OR SUPERPLASTICIZERS ARE ALLOWED IN THE MIX TO ASSIST IN WORKABILITY AND PUMPING OPERATIONS, AND MIX-DESIGN MAKE-UP WATER MAY BE POST SUPPLEMENTED AT THE SITE. TESTING FIRM MUST MEASURE AND REPORT.
- 19. CONCRETE CEMENT SHALL BE TYPE I OR II PORTLAND CEMENT, MAXIMUM AGGREGATE SIZE IN FOUNDATION CONCRETE SHALL BE 11/2". MAXIMUM AGGREGATE SIZE IN SLAB CONCRETE SHALL BE ¾".
- 4. GROUT TO FILL CELLS AND BOND BEAMS SHALL CONFORM TO ASTM C476, FINE TYPE, AND 20. ALL CONCRETE SHALL BE READI-MIXED AT PLANT COMPLYING WITH ASTM C94 AND ASTM C1116. SITE MIXING IS NOT ALLOWED.
 - 21. CONCRETE SHALL BE PLACED UPON A MINIMUM 12" THICK BED OF 3/4" CRUSHED, WASHED STONE COMPACTED TO A STABLE AND UNYIELDING STATE, IN 6" (MAX.) LIFTS. ALL STONE BED SUB-BASE COMPACTED MATERIALS SHALL BE PLACED ATOP SPECIFIED FILTER FABRIC AND COMPACTED STRUCTURAL FILL AND PROOF-ROLLED, ACCEPTABLE (AS DETERMINED BY GEOTECHNICAL ENGINEER) MATERIALS.
 - 22. ALL CONCRETE SHALL BE REINFORCED AS SHOWN ON THE DRAWINGS. PROVIDE SUFFICIENT CHAIR OR SUPPORT BARS AS NECESSARY TO PROPERLY POSITION REINFORCING STEEL. PULL UP OF BARS OR MESH, OR UNSUPPORTED BARS OR MESH WILL NOT BE ALLOWED. "WET STICKING" OF BARS WILL NOT BE ALLOWED.
 - 23. SUBMIT ALL GEOTECHNICAL ENGINEER REPORTS TO THE ENGINEER FOR APPROVAL PRIOR TO COMMENCING WITH PLACEMENT OF SUB-BASE MATERIALS, FILTER FABRIC, REINFORCING STEEL, ETC. SUBMIT CONCRETE TEST RESULTS FROM TESTING AGENCY TO THE ENGINEER. OBTAIN ENGINEER'S APPROVAL OF ALL SUBMITTALS PRIOR TO COMMENCING WITH WORK.
 - 24. ALL CONCRETE SHALL BE POURED TO THE SPECIFIED THICKNESS AND REINFORCED AS SHOWN ON THE DRAWINGS. WELDED WIRE MESH REINFORCEMENT SHALL CONFORM TO A.S.T.M. A185, AND SHALL LAP 6" MINIMUM OR ONE SPACE, WHICHEVER IS LARGER, AND SHALL BE WIRED TOGETHER. WELDED WIRE USED AT EXTERIOR SLABS SHALL BE EPOXY COATED TO CONFORM TO LATEST CRSI SPECIFICATIONS, AND SHALL BE SET 2 INCHES BELOW TOP SURFACE OF SLAB. IT IS REQUIRED THAT CHAIR BARS BE USED TO MAINTAIN PROPER LOCATION OF WELDED WIRE MESH.

GRANITE JOINTS (STAIRS):

- GRANITE BLOCK JOINT SEALANT SHALL BE A HIGH DUROMETER, MULTI-COMPONENT URETHANE SEALANT WITH A MINIMUM SHORE A HARDNESS OF 40. SEALANT SHALL CONFORM TO ASTM C920, TYPE M, GRADE NS, SUCH AS SIKAFLEX-2C-NS-TG OR APPROVED EQUAL. SEALANT WILL BE INSTALLED OVER MORTAR JOINT.
- 2. COLOR(S) SHALL BE SELECTED BY THE OWNER FROM THE APPROVED MANUFACTURER'S PREMIUM COLOR CHART.
- 3. CLEANERS AND PRIMERS SHALL BE AS RECOMMENDED BY THE MANUFACTURER OF THE CAULKING.
- 4. BACKER ROD SHALL BE CONTINUOUS LENGTH, CLOSED CELL POLYETHYLENE FOAM, AS RECOMMENDED BY THE SEALANT MANUFACTURER. BACKER ROD SHALL BE COMPRESSIBLE, RESILIENT, NON-WAXING, NON-EXTRUDING AND NON-STAINING. BACKER ROD SHALL BE OF SUFFICIENT SIZE TO BE COMPRESSED 30% OF MAXIMUM JOINT WIDTH AND SHALL BE TOTALLY COMPATIBLE WITH THE SEALANT, PRIMER AND SUBSTRATES. BACKERS SHALL CONFORM TO THE REQUIREMENTS OF ASTM C 962 - TYPE A, ASTM D 1622, ASTM D 1623 AND ASTM D 5249.
- 5. BOND BREAKER TAPE SHALL BE SELF-ADHESIVE POLYETHYLENE TAPE AS RECOMMENDED BY THE SEALANT MANUFACTURER.
- 6. REMOVE EXISTING SEALANTS OR REMNANTS OF MORTAR FROM ALL SURFACES TO RECEIVE NEW SEALANTS. WIRE BRUSH, SCRAPE. CHIP OR GRIND SUBSTRATE AND ADJACENT SURFACES TO RECEIVE SEALANT AS REQUIRED TO REMOVE ALL TRACES OF EXISTING SEALANTS MORTAR AND ALL CONTAMINANTS WHICH WILL EFFECT BONDING. OBTAIN SURFACES ACCEPTABLE BY THE SEALANT MANUFACTURER FOR INSTALLATION OF NEW SEALANTS. GRANITE SURFACES MUST HAVE AN OPEN CAPILLARY, MILDLY ROUGHENED, DUST FREE, GROUND SURFACE PRIOR TO APPLYING PRIMER AND SEALANTS.
- 7. FOLLOWING REMOVAL OF EXISTING SEALANTS AND PREPARATION OF BONDING SURFACES, CLEAN BONDING SURFACES WITH TWO APPLICATIONS OF THE MANUFACTURER'S RECOMMENDED CLEANING SOLUTION. APPLY SOLUTION WITH BRUSHES AND WIPE WITH CLEAN WHITE RAGS.
- JOINT PRIMER SHALL BE APPLIED TO PROPERLY PREPARED, CLEANED AND DRIED SUBSTRATES. PRIMER SHALL BE AS PROVIDED BY THE SEALANT MANUFACTURER FOR EACH SUBSTRATE AND SHALL BE COMPLETELY COMPATIBLE WITH THE EXISTING MATERIALS AND PROPOSED SEALANTS AND ACCESSORIES.
- 9. PRIMER SHALL BE APPLIED AND ALLOWED TO DRY PRIOR TO THE APPLICATION OF BOND BREAKER AND SEALANT.
- 10. SEALANT JOINTS SHALL BE INSTALLED IN ACCORDANCE WITH THE MANUFACTURER'S WIDTH TO DEPTH RATIO REQUIREMENTS AND AS DETAILED.
- 11. SEALANT SHALL BE APPLIED TO CLEAN, DRY JOINTS BY KNIFE, TROWEL, MANUAL OR AIR PRESSURE
- 12. SEALANT SHALL BE FORCED INTO THE JOINT TO COMPLETELY FILL THE VOID. FORCE SEALANT INTO THE JOINT AND AGAINST THE SIDES OF THE JOINT. AVOID PULLING SEALANT FROM THE SIDES OF
- 13. ALL JOINT SEALANT SHALL BE IMMEDIATELY TOOLED TO ASSURE FULL ADHESION. SEALANT SHALL BE DRY TOOLED STRAIGHT, UNIFORM, SMOOTH, AND NEATLY FINISHED TO THE PROFILES DETAILED. NO SOAPS, WETTING OR SLICKING AGENTS WILL BE ALLOWED.
- 14. USE CAUTION NOT TO DISTURB THE EXISTING SEALANT JOINTS AFTER INSTALLATION.

QUALITY CONTROL (STAIRS)

CAULKING GUNS USING PROPER NOZZLE SIZES.

- 1. FOR ALL CONSTRUCTION MATERIALS SUBMIT AS PER SECTION 01300: LIST OF MATERIALS AND MANUFACTURERS, CATALOG DATA, MANUFACTURER'S INSTRUCTIONS, MATERIALS SAFETY DATA SHEETS, AND SHOP DRAWINGS.
- 2. SUBMIT A CONSTRUCTION SCHEDULE.
- THE CONTRACTOR SHALL PROVIDE GRANITE SAMPLE FOR OWNER APPROVAL AS TO ACCEPTABILITY OF COLOR, TEXTURE AND APPEARANCE MATCH WITH THE EXISTING CONSTRUCTION.
- 4. THE CONTRACTOR SHALL PROVIDE A COLOR CHART FROM THE APPROVED MORTAR MANUFACTURER. COLOR(S) SHALL BE SELECTED BY OWNER.
- COORDINATE WORK WITH THAT OF OTHER SECTIONS AFFECTING, AFFECTED BY, THIS WORK, AS NECESSARY TO ASSURE THE STEADY PROGRESS OF THE WORK UNDER THE CONTRACT.
- DO ALL CUTTING AND DRILLING TO ACCOMMODATE WORK OF OTHER SECTIONS, AS EXPRESSLY INDICATED AND AS REASONABLY INFERRED FROM CONTRACT DOCUMENTS AND SPECIFICATIONS, OR REQUIRED FOR THE PROPER COMPLETION OF THE WORK.

7. CLEANING:

- 7.1. AFTER POINTING GRANITE, WORK SHALL BE CAREFULLY CLEANED, REMOVING ALL DIRT, EXCESS MORTAR, STAINS AND OTHER DEFACEMENTS.
- 7.2. MILD ABRASIVE CLEANERS THAT CONTAIN NO HARSH OR CAUSTIC INGREDIENTS MAY BE USED, WITH FIBER BROOMS OR BRUSHES AND CLEAR WATER. WIRE BRUSHES, STEEL WOOL, AND ACIDS OR OTHER SOLUTIONS WHICH MAY CAUSE DISCOLORATION ARE EXPRESSLY PROHIBITED.
- 7.3. UPON COMPLETION OF GRANITE WORK, SURFACES SHALL BE LEFT IN A CLEAN, UNSOILED CONDITION, ACCEPTABLE TO THE ARCHITECT AND OWNER.
- 7.4. CLEAN THE ENTIRE WORK AREA OF DEBRIS PRIOR TO DEPARTING FROM THE WORK SITE ON A DAILY BASIS TO THE SATISFACTION OF THE OWNER.
- 7.5. ALL BUILDING COMPONENTS, PAVED AND LANDSCAPED AREAS SHALL BE THOROUGHLY CLEANED OF ALL DEBRIS AND MATERIALS OF CONSTRUCTION. ALL CLEAN-UP SHALL BE TO THE SATISFACTION OF THE OWNER, AND SHALL BE PERFORMED ON A DAILY BASIS.
- 7.6. ANY LANDSCAPED, PAVED AREAS, INTERIOR OR EXTERIOR BUILDING COMPONENTS DAMAGED DUE TO THE WORK SHALL BE REPAIRED OR REPLACED, TO THE SATISFACTION OF AND AT NO ADDITIONAL COST TO THE OWNER.

- 8.1. GRANITE WORK SHALL BE PROPERLY AND ADEQUATELY PROTECTED UNDER THE RESPONSIBILITY OF THE CONTRACTOR UNTIL FINAL ACCEPTANCE OF THE PROJECT BY OWNER.
- 8.2. AFTER THE GRANITE WORK HAS BEEN INSTALLED, IT SHALL BE PROPERLY AND ADEQUATELY PROTECTED FROM DAMAGE. BOXING OR OTHER SUITABLE PROTECTION SHALL BE PROVIDED BY CONTRACTOR WHEREVER REQUIRED. HOWEVER, NO LUMBER WHICH MAY STAIN OR DEFACE THE GRANITE SHALL BE USED. NAILS SHALL BE HIGH-QUALITY GALVANIZED OR NON-RUSTING.

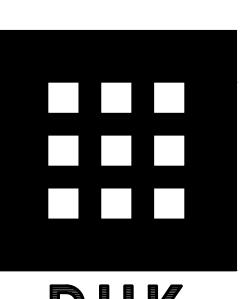
GEOTECHNICAL NOTES (STAIRS):

- 1. THE SITE MAY POTENTIALLY CONTAIN ORGANICS, FILL, BURIED WOOD AND TOPSOIL, AND OTHER UNSUITABLE SOILS AT SOME LOCATIONS. OVER-EXCAVATION TO REMOVE THESE UNACCEPTABLE SOILS MAY BE REQUIRED FOR THE PROJECT CONCRETE AND SUB-BASE MATERIAL PLACEMENT. COMPACTED STRUCTURAL FILL AND COMPACTED STONE SUB-BASE MATERIALS WILL BE REQUIRED TO BE PLACED IN CONTROLLED LOOSE LIFTS AT ALL BACKFILLING LOCATIONS.
- 2. THE CONTRACTOR SHALL PROVIDE AMPLE AND CONTINUOUS SITE DEWATERING DURING THE ENTIRE CONSTRUCTION PHASE OF THIS PROJECT AS REQUIRED. OBTAIN TOWN APPROVAL FOR DISCHARGE METHODS.
- 3. THE ENTIRE CONSTRUCTION PHASE OF THIS PROJECT MUST INCLUDE THE FIELD REVIEW SERVICES OF A MASSACHUSETTS LICENSED GEOTECHNICAL ENGINEER, OR THEIR REPRESENTATIVE, TO OBSERVE, REVIEW, MAKE CORRECTIVE ACTIONS, REPORT, AND CERTIFY THAT ALL EARTH-WORK CONFORMS WITH THE PROJECT REQUIREMENTS. THE LICENSED GEOTECHNICAL ENGINEER'S FIELD SERVICES MUST INCLUDE THE FOLLOWING AS A MINIMUM:
 - OBSERVE REPRESENTATIVE SOIL OVER-EXCAVATION PROCEDURES TO VERIFY THAT ALL UNSUITABLE MATERIALS HAVE BEEN SUFFICIENTLY REMOVED.
 - VERIFY THAT THE UNDERLYING SOILS (AFTER REMOVAL OF UNSUITABLE MATERIALS) ARE ACCEPTABLE FOR USE AS THE SUB-BASE TO STRUCTURAL FILL. CRUSHED STONE AND FOUNDATION PLACEMENT
 - OBSERVE REPRESENTATIVE PROOF—ROLLING OPERATIONS AND PROCEDURES OF UNDISTURBED OR VIRGIN, ON SITE SOIL MATERIALS, IF FOUND ACCEPTABLE TO REMAIN.
 - OBSERVE LOOSE LIFT PLACEMENT AND CONTROLLED COMPACTION OF STRUCTURAL FILL AND CRUSHED STONE SUB-BASE AND BACKFILL MATERIALS.
 - PROVIDE RECOMMENDATIONS VERIFYING COMPLIANCE AND ADEQUACY OF SITE DEWATERING.
 - A FIELD REPORT FOR EACH SITE VISIT PERFORMED BY THE LICENSED GEOTECHNICAL ENGINEER OR THEIR REPRESENTATIVE SHALL BE WRITTEN AND SUBMITTED TO THE OWNER AND ENGINEER. THE REPORT SHALL INCLUDE THE DATE, PERSONNEL PRESENT AT THE SITE, WEATHER, TIME, PURPOSE OF SITE VISIT, AREAS INCLUDED IN THE SITE OBSERVATIONS WITH PLAN SKETCH, CORRECTIVE ACTIONS RECOMMENDED, AND RESULTS OF OBSERVED WORK.
 - UPON COMPLETION OF THE GEOTECHNICAL ENGINEER'S FIELD OBSERVATION WORK, AND SUBMISSION OF ALL FIELD REPORTS AND SITE TESTING (IF REQUIRED), THE GEOTECHNICAL ENGINEER MUST SUBMIT AN ENGINEER'S SEALED FINAL LETTER OF CERTIFICATION TO STATE THAT THE GEOTECHNICAL RELATED CONSTRUCTION HAS MET THE REQUIREMENTS OF THE PROJECT.
 - THE GEOTECHNICAL ENGINEER SHALL INCLUDE FIELD TESTING TO VERIFY COMPACTION AND VALIDATION OF STRUCTURAL FILL, GRANULAR FILL, CRUSHED STONE AND OTHER BORROWED SOIL MATERIALS FOR THE PROJECT. THE GEOTECHNICAL ENGINEER SHALL SUBMIT THEIR INTENDED TESTING FREQUENCY AND PROTOCOL TO THE OWNER AND ENGINEER FOR ACCEPTANCE PRIOR TO INITIATING THE WORK.
- ALL SLABS AND FOUNDATIONS SHALL BEAR UPON THE SPECIFIED, COMPACTED, CRUSHED STONE LAYER OVER COMPACTED STRUCTURAL FILL AFTER REMOVAL OF ALL UNSUITABLE MATERIALS. IF THE EXISTING MATERIALS ARE FOUND TO BE ACCEPTABLE BY THE GEOTECHNICAL ENGINEER IN LIEU OF USING THE STRUCTURAL FILL MATERIALS, THIS ACCEPTANCE MUST BE SUBMITTED TO THE OWNER AND STRUCTURAL ENGINEER IN WRITING BEFORE PLACING ANY SLAB OR FOUNDATIONS. ALL BACKFILLED STRUCTURAL FILL MATERIALS SHALL BE COMPACTED TO 95% (MINIMUM) MODIFIED PROCTOR, ASTM D1557, MAXIMUM DRY DENSITY
- 6. THE OWNER IS REQUIRED TO HIRE AND PAY FOR ALL GEOTECHNICAL ENGINEERING SERVICES THROUGH THE DURATION OF THAT PORTION OF THE PROJECT.
- 7. THE CONTRACTOR IS REQUIRED TO CONTACT, COORDINATE, SCHEDULE, AND ASSIST THE GEOTECHNICAL ENGINEER FOR ALL FIELD REVIEW WORK.
- 8. CRUSHED STONE FOR USE UNDER CONCRETE SLABS AND FOUNDATIONS SHALL BE 3/4" CRUSHED, ANGULAR, WASHED STONE OF GRANITE (OR OTHER APPROVED) QUARRY. ALL CRUSHED STONE SHALL BE PLACED IN A MAXIMUM 4" THICK LOOSE LIFT, AND THEN COMPACTED TO A STABLE. NON-YIELDING STATE.

SIEVE (ASTM D422)	PERCENT PASSING BY WEIGHT
1 INCH	100
3/4-INCH	90 - 100
1/2-INCH	10 - 50
3/8-INCH	0 - 20
NO. 4	0 - 5

9. STRUCTURAL FILL SHALL BE FREE FROM ICE, SNOW, ROOTS, SOD, RUBBISH, DEBRIS AND OTHER DELETERIOUS OR ORGANIC MATTER AND CONFORM TO THE FOLLOWING GRADATION:

SIEVE (ASTM D422)	PERCENT PASSING BY WEIGHT
3 INCH	100
1/2-INCH	50 - 85
NO. 4	40 - 75
NO. 40	10 - 35
NO. 200	0 - 8



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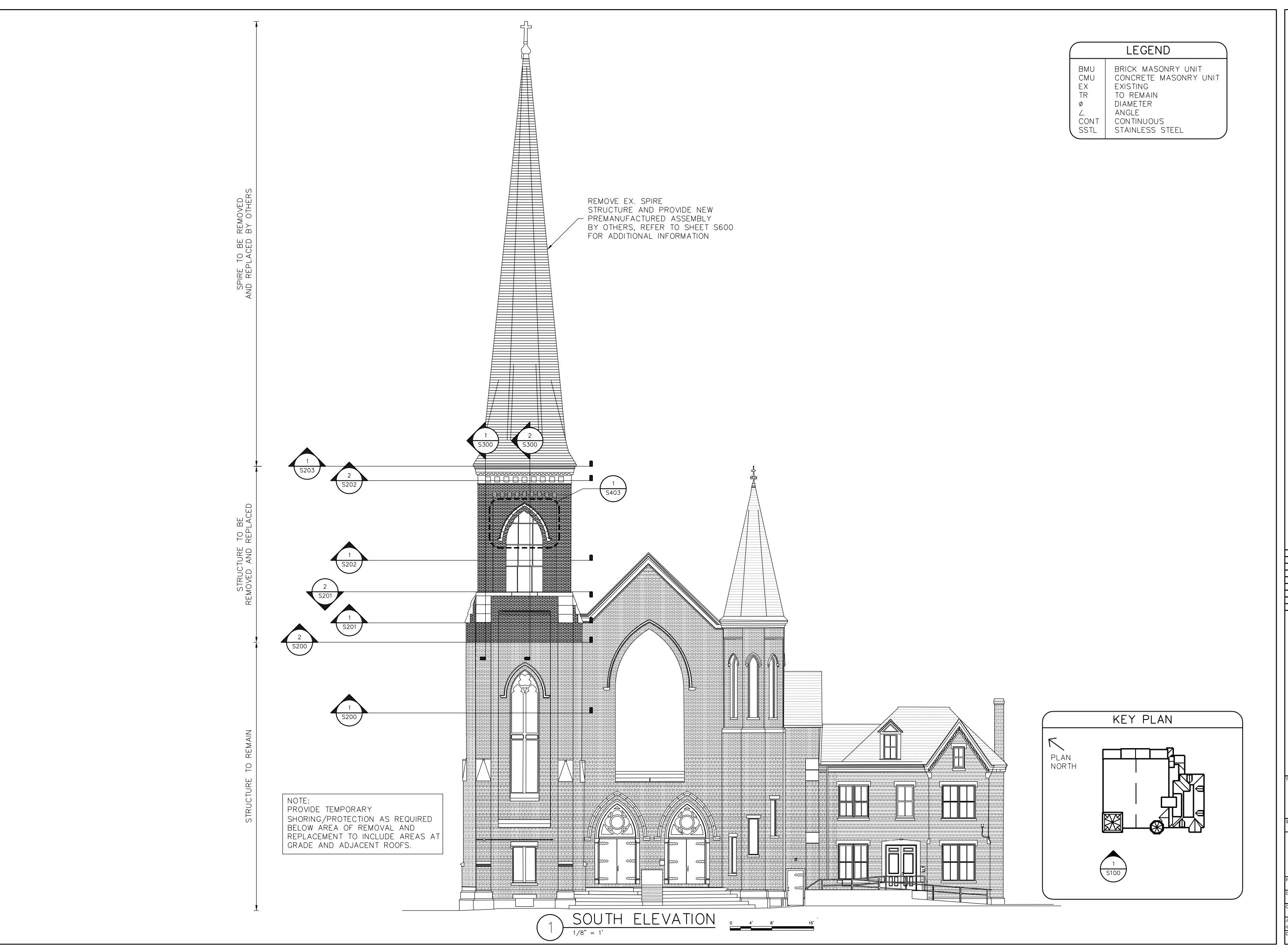
TECHNICAL NOTES (CONT.)

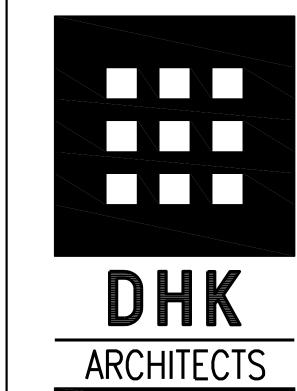


KPB EWM SMF MDF CM CGM

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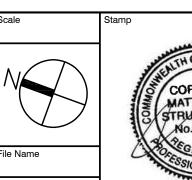
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ELEVATION



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PDA CM CGM

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GALE Job No. 832681

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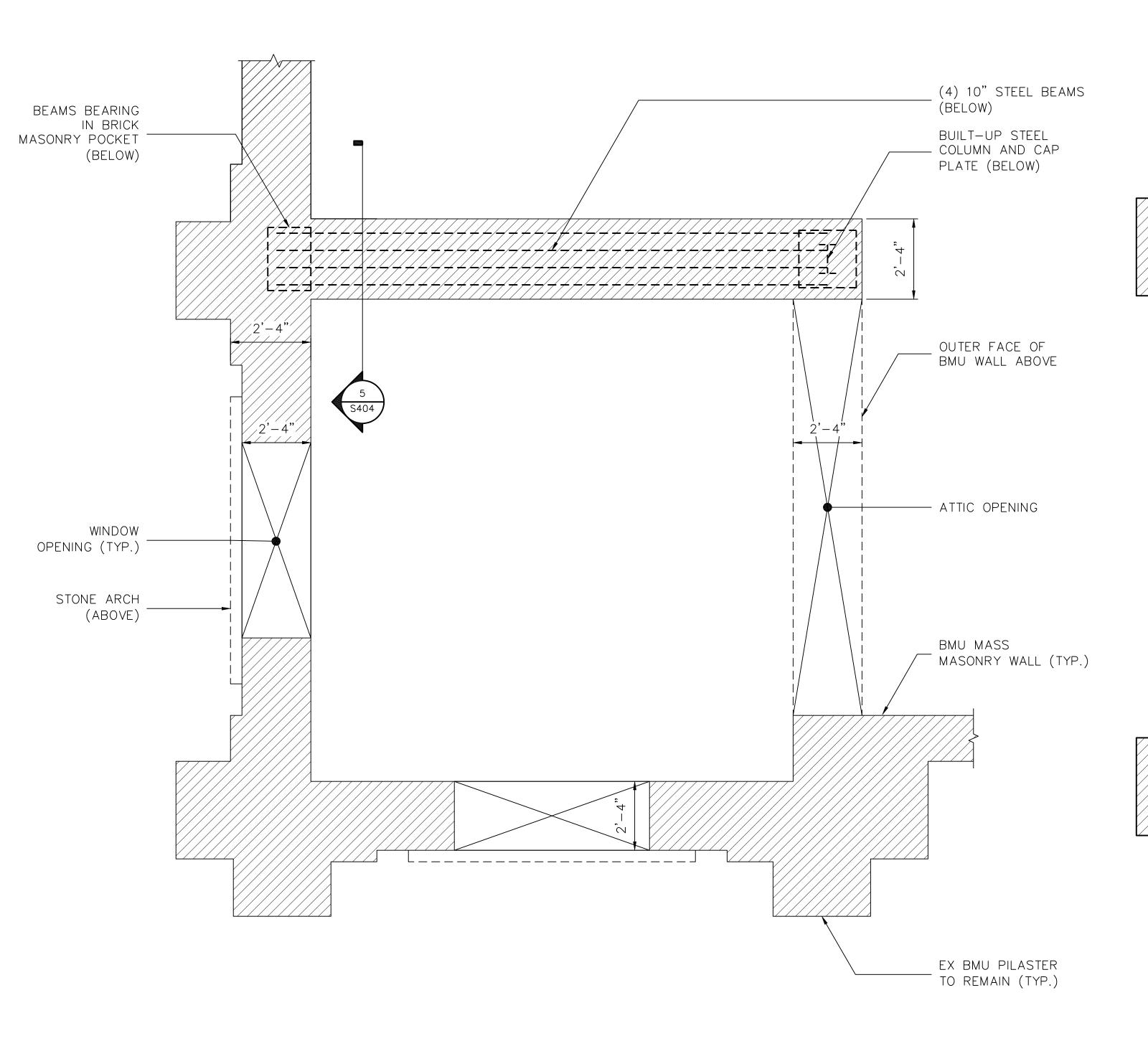
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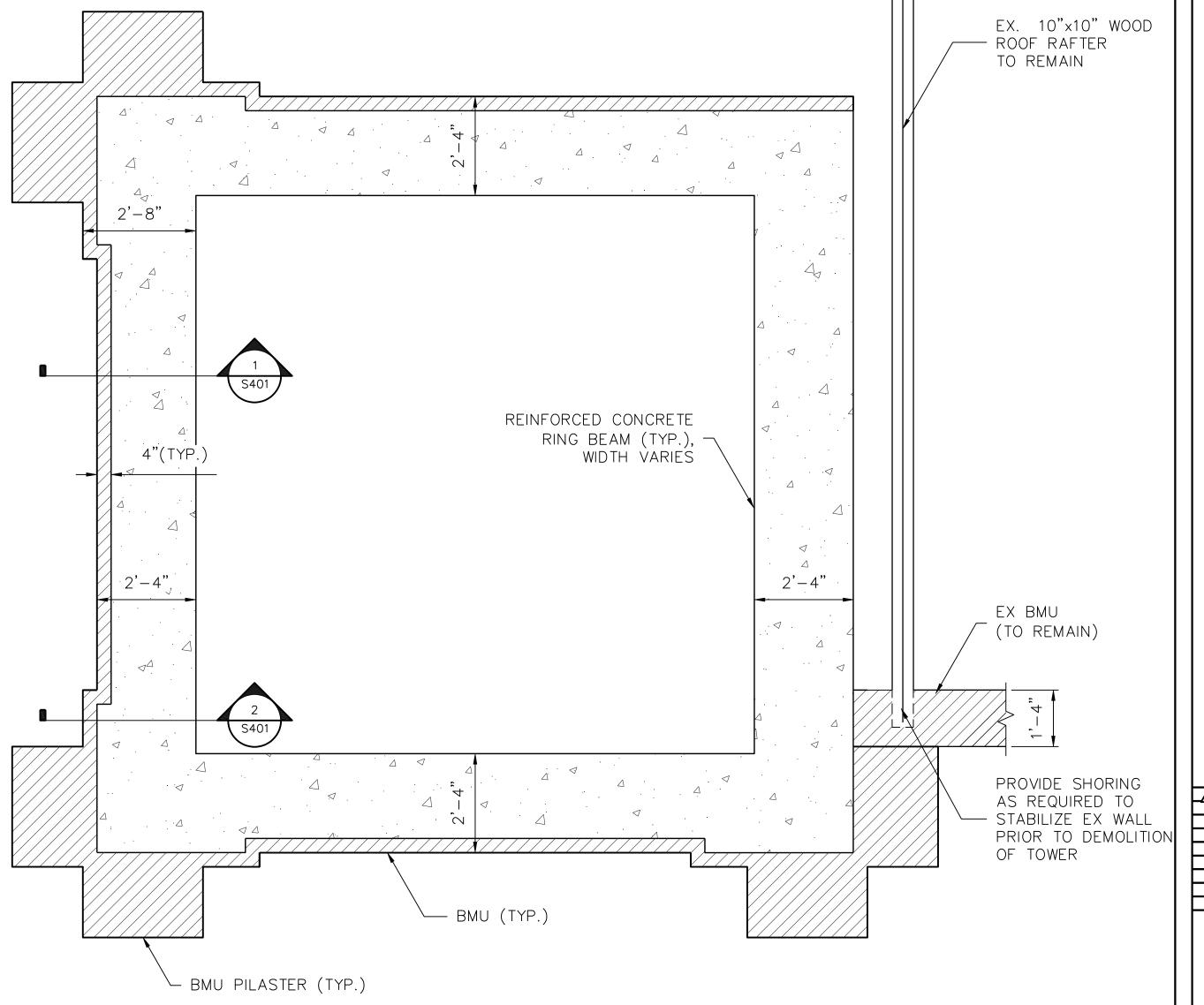
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TOWER PLAN — BELOW EX STEEL LINTEL

ALL ITEMS ARE EXISTING (EX) TO REMAIN

TOWER PLAN @ LOWER RING BEAM

ALL ITEMS ARE NEW UNLESS INDICATED AS EXISTING (EX)

GENERAL NOTES

- REFER TO ARCHITECTURAL DRAWINGS FOR DIMENSION OF NEW MASONRY WALLS AND REBUILT PILASTERS.
- REFER TO SPECIFICATIONS FOR CONSTRUCTION OF NEW MASONRY WALLS.



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TOWER PLANS





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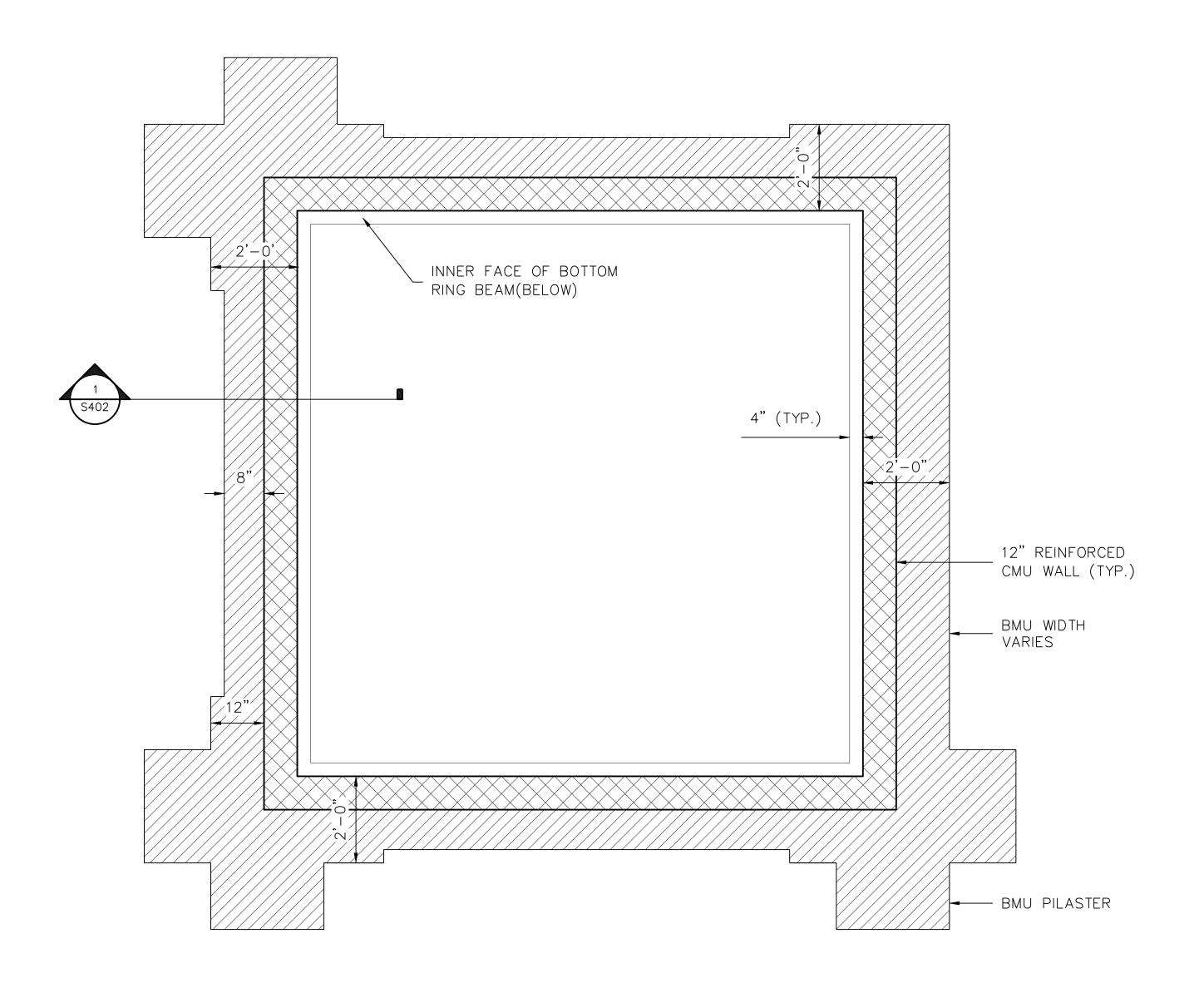
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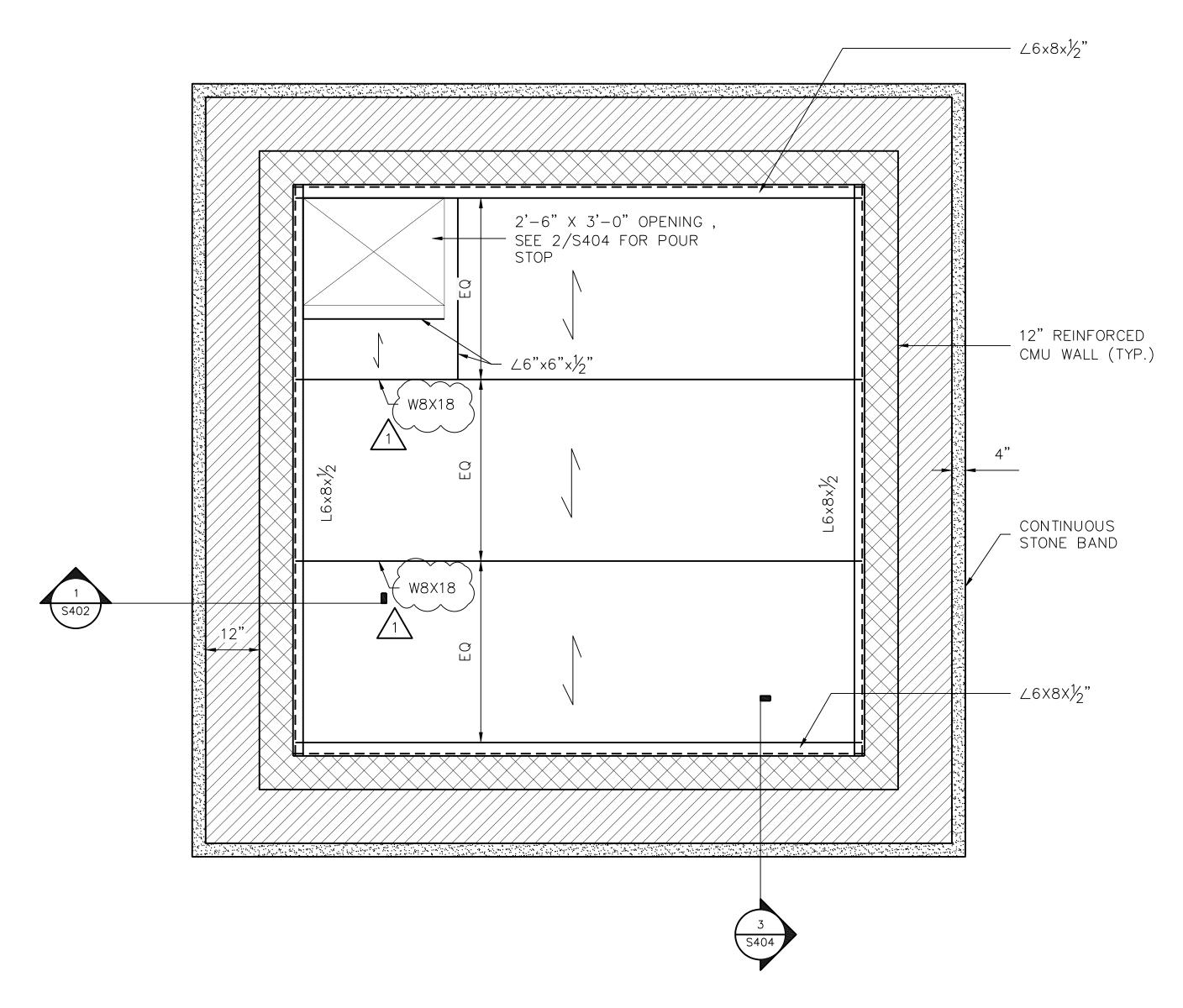
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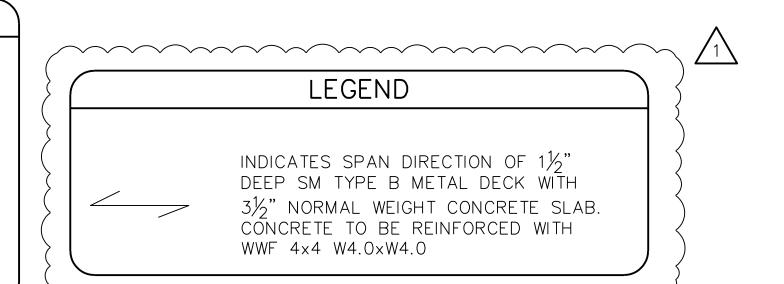


TOWER PLAN - BELOW BELFRY FLOOR



GENERAL NOTES

- REFER TO ARCHITECTURAL DRAWINGS FOR DIMENSION OF NEW MASONRY WALLS AND REBUILT PILASTERS.
- REFER TO SPECIFICATIONS FOR CONSTRUCTION OF NEW MASONRY WALLS.
- SPRAY APPLIED FIREPROOFING TO BE PROVIDED AT STEEL FRAMING.
- REFER TO ARCHITECTURAL DRAWINGS FOR ELEV. OF TOP OF SLAB.





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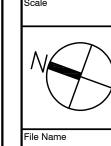
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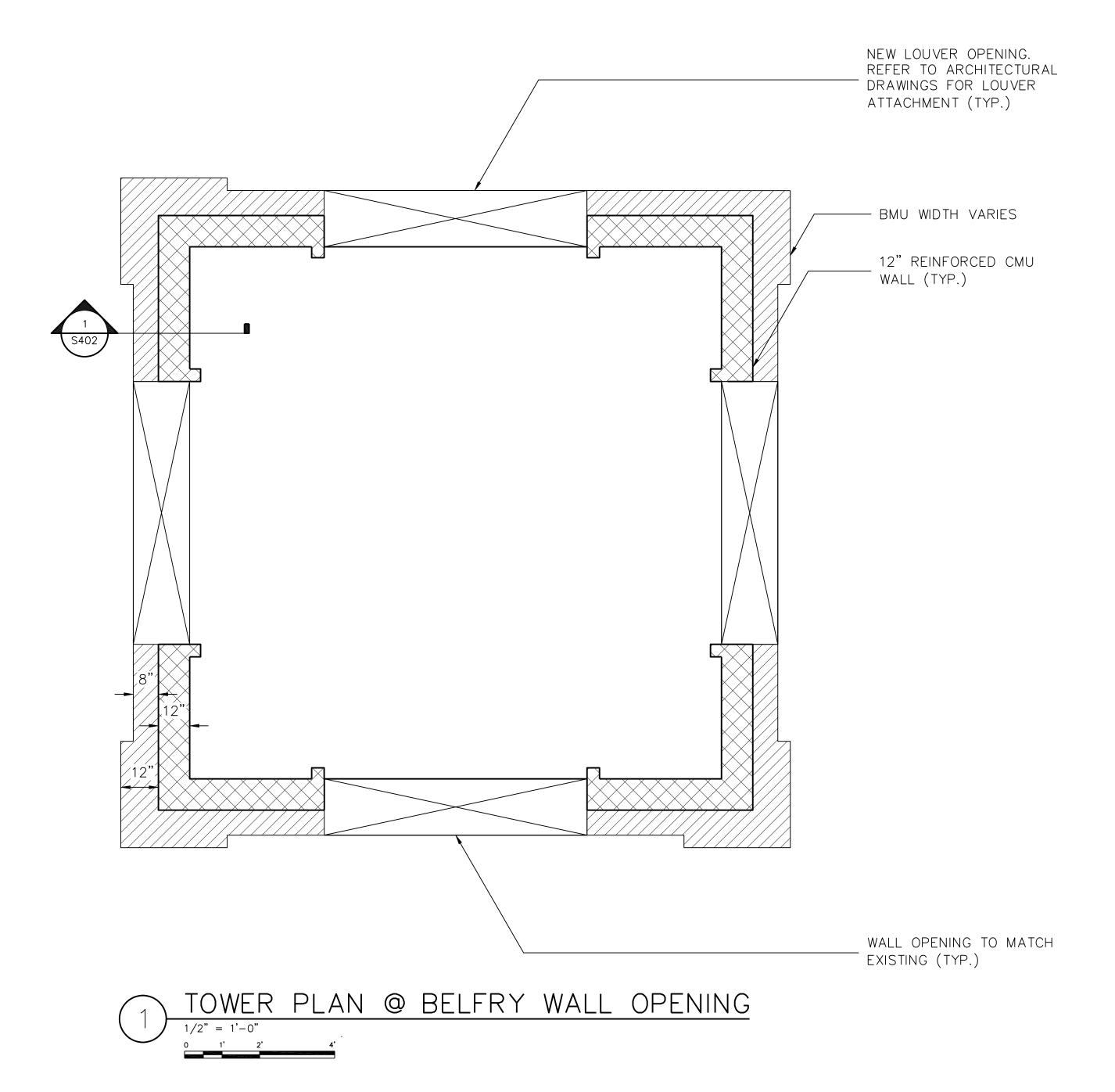
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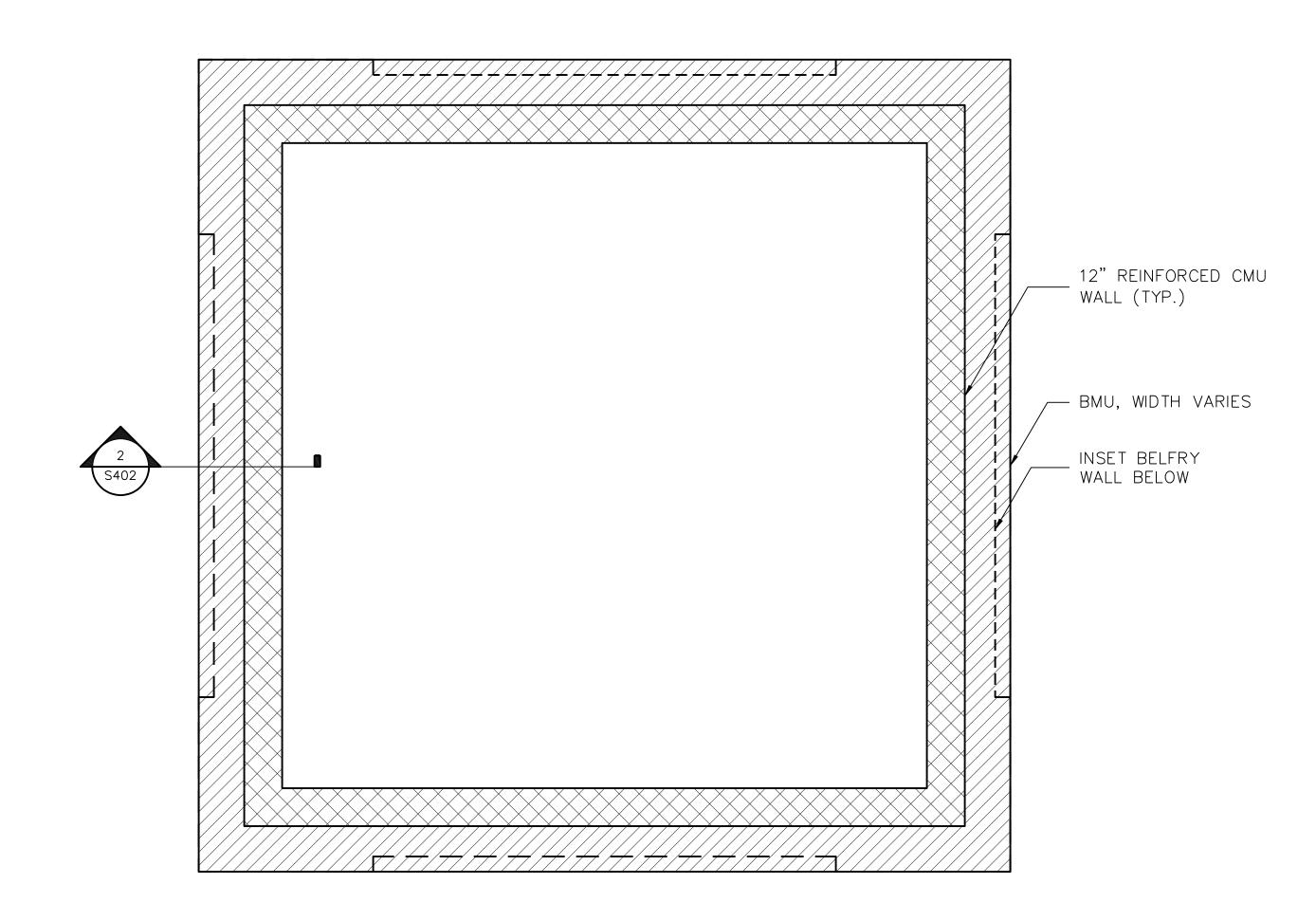




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GENERAL NOTES

- REFER TO ARCHITECTURAL DRAWINGS FOR DIMENSION OF NEW MASONRY WALLS AND REBUILT PILASTERS.
- REFER TO SPECIFICATIONS FOR CONSTRUCTION OF NEW MASONRY WALLS.



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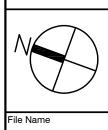
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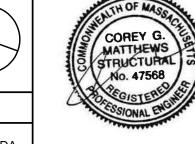
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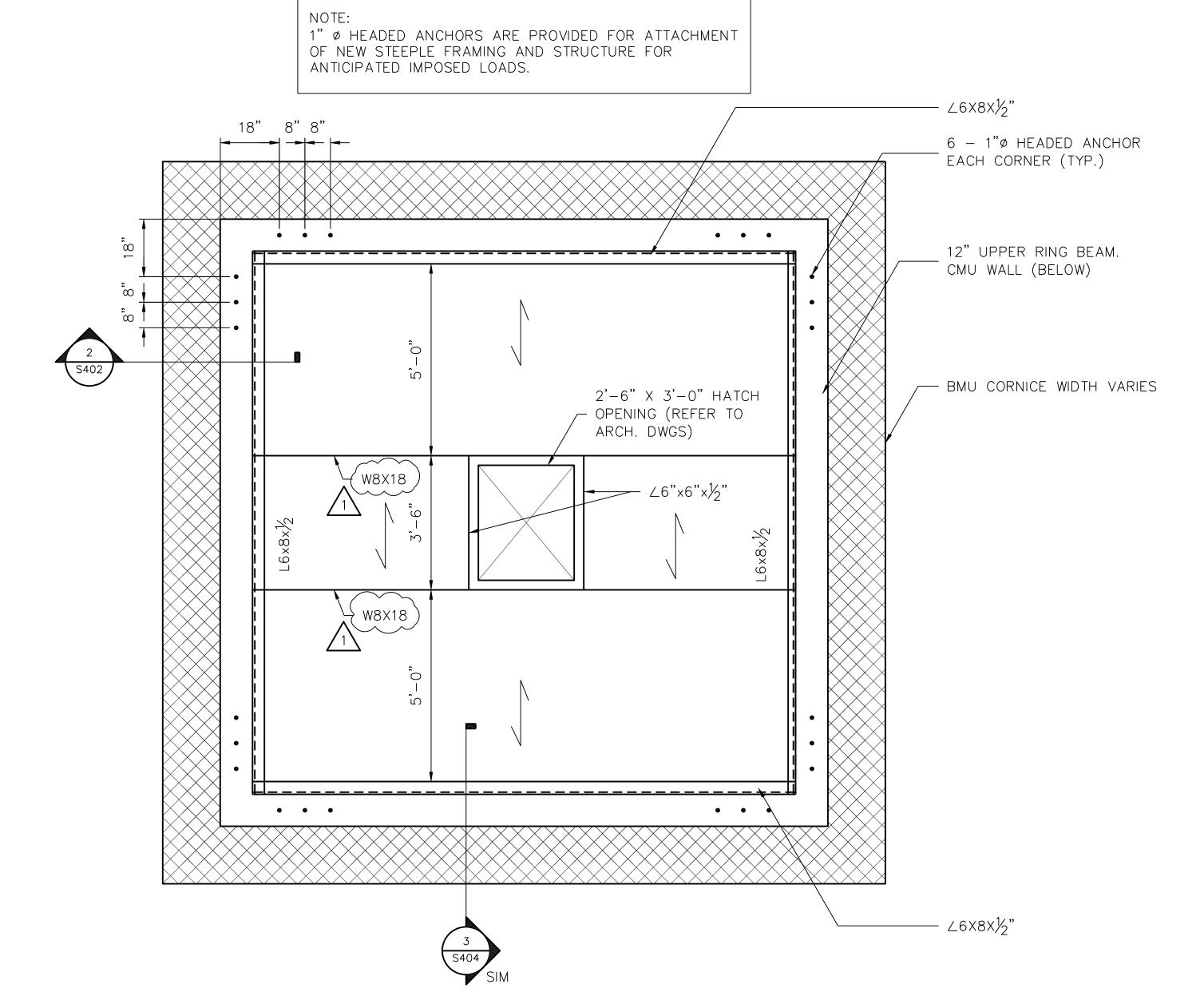




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TOWER PLAN @ UPPER RING BEAM AND ROOF SLAB

GENERAL NOTES

- REFER TO ARCHITECTURAL DRAWINGS FOR DIMENSION OF NEW MASONRY WALLS AND REBUILT PILASTERS.
- REFER TO SPECIFICATIONS FOR CONSTRUCTION OF NEW MASONRY WALLS.
- SPRAY APPLIED FIREPROOFING TO BE PROVIDED AT STEEL FRAMING.
- REFER TO ARCHITECTURAL DRAWINGS FOR ELEV. OF TOP OF SLAB.

LEGEND

INDICATES SPAN DIRECTION OF 1½"
DEEP SM TYPE B METAL DECK WITH
3½" NORMAL WEIGHT CONCRETE SLAB.
CONCRETE TO BE REINFORCED WITH
WWF 4×4 W4.0×W4.0

ARCHITECTS

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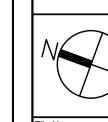
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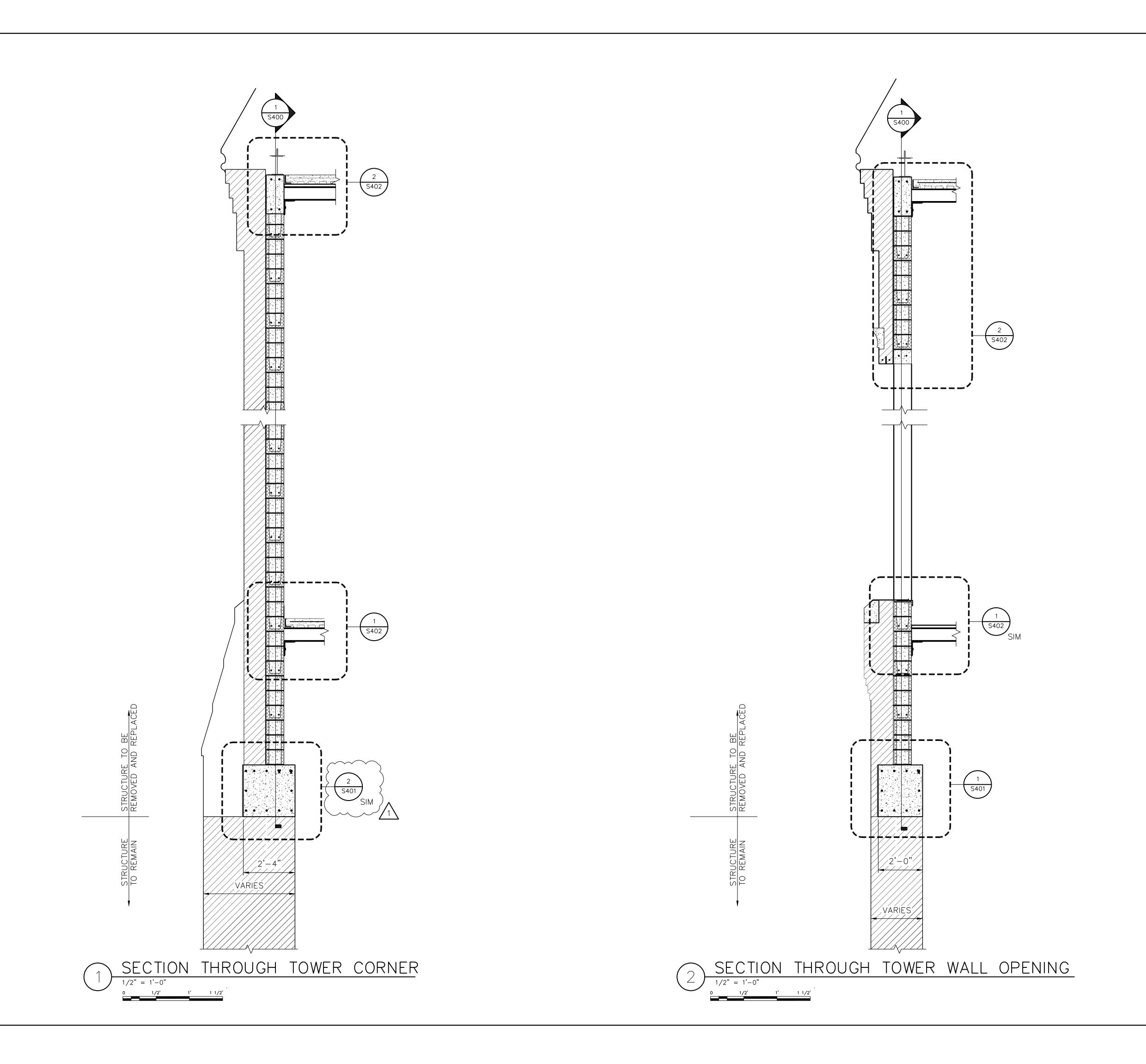




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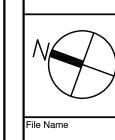
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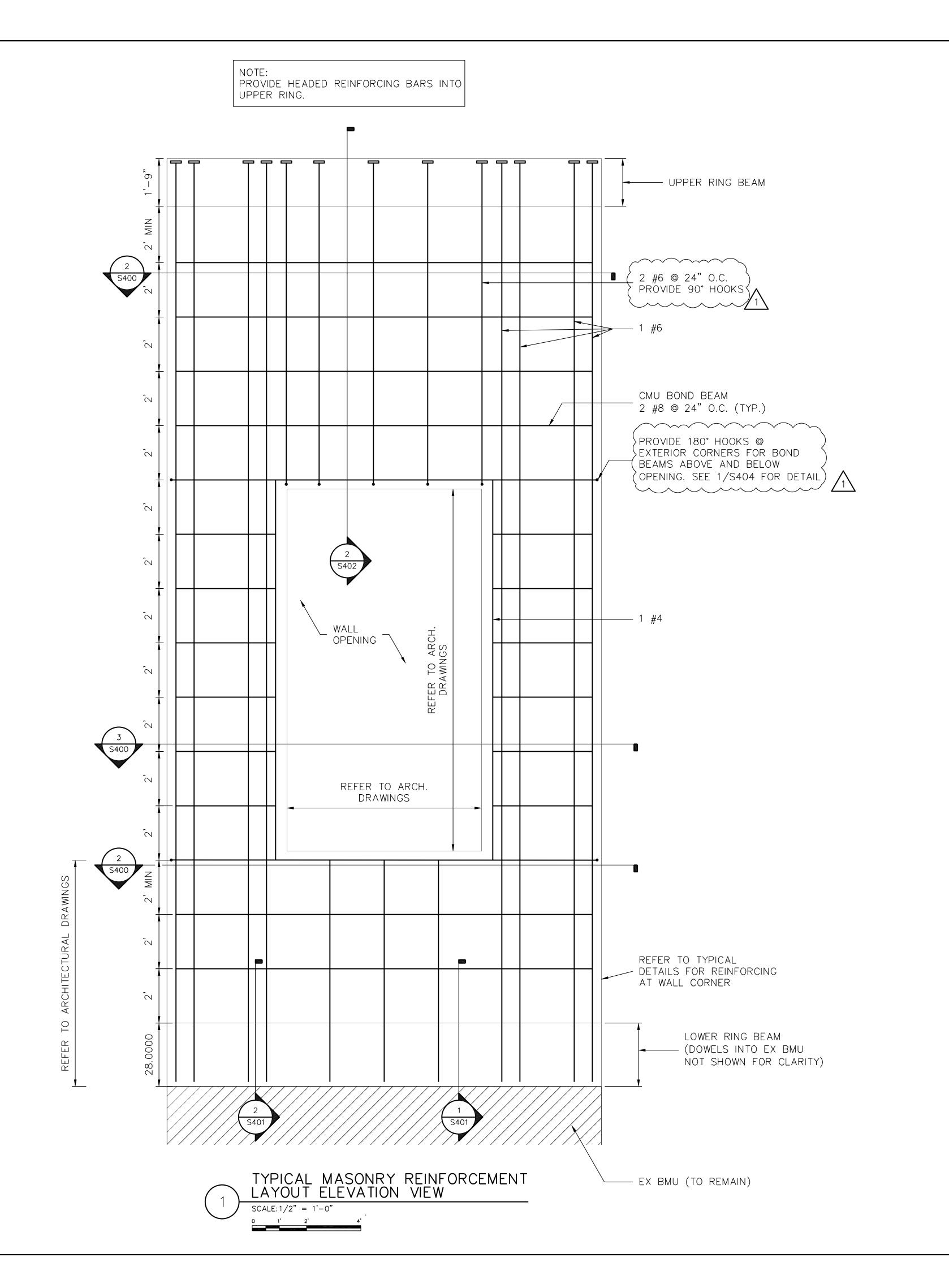
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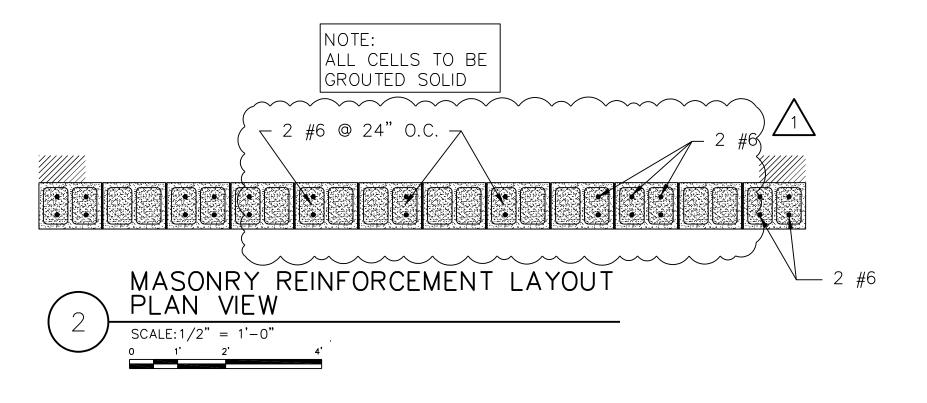


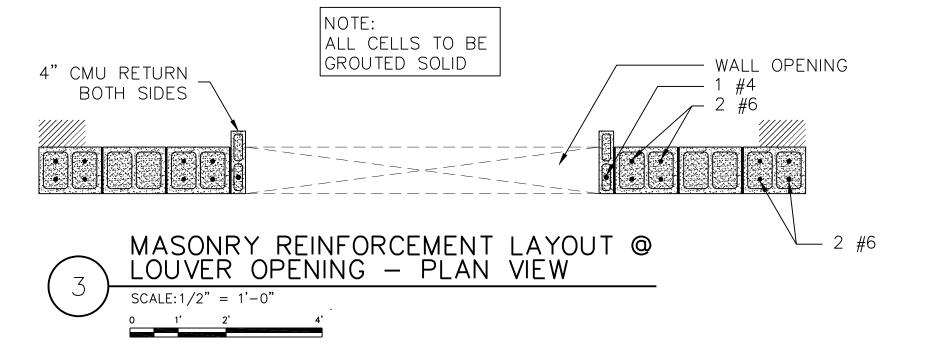


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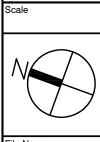
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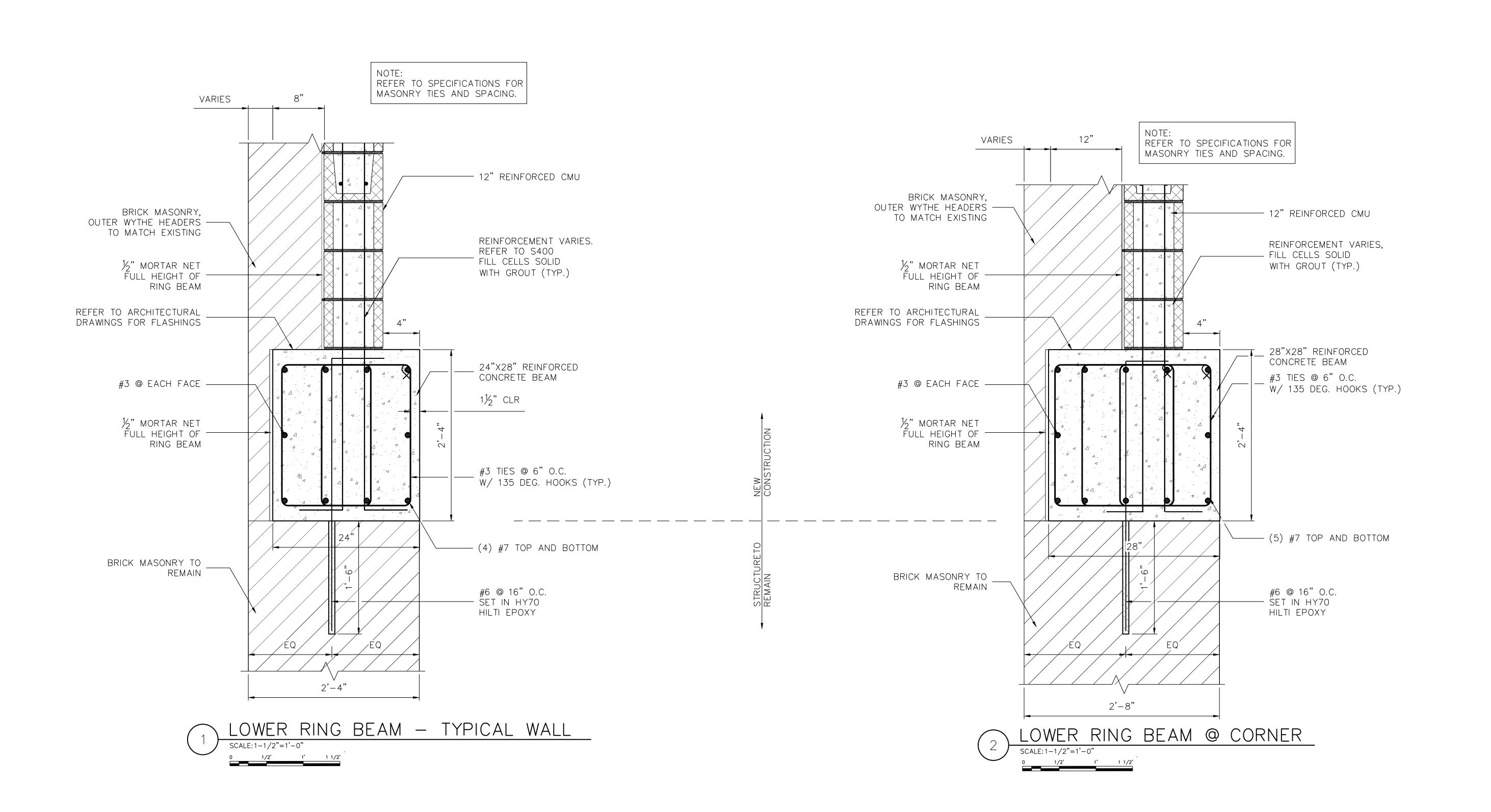
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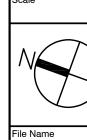
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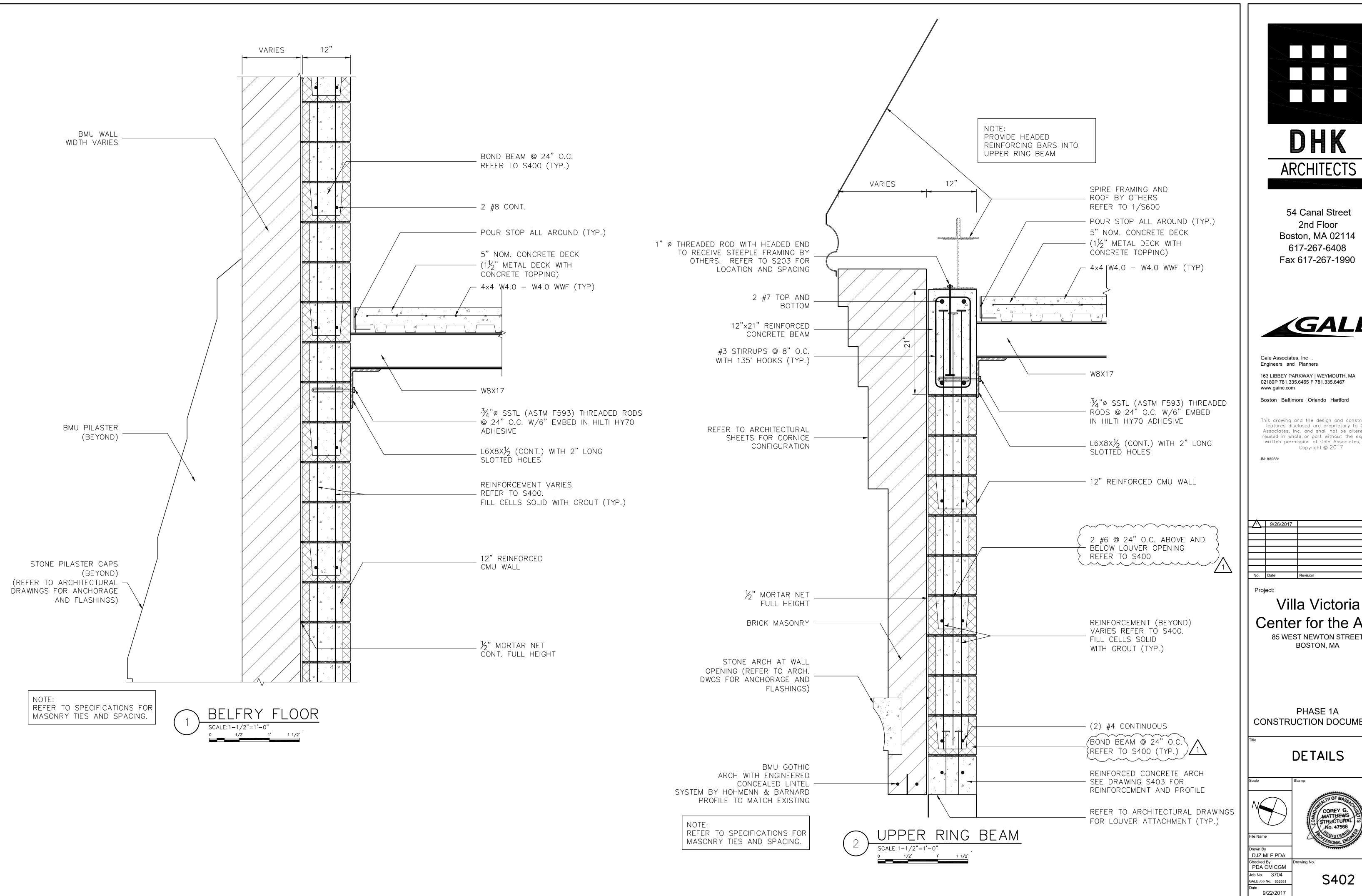
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COREY G.
MATTHEWS
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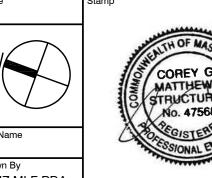
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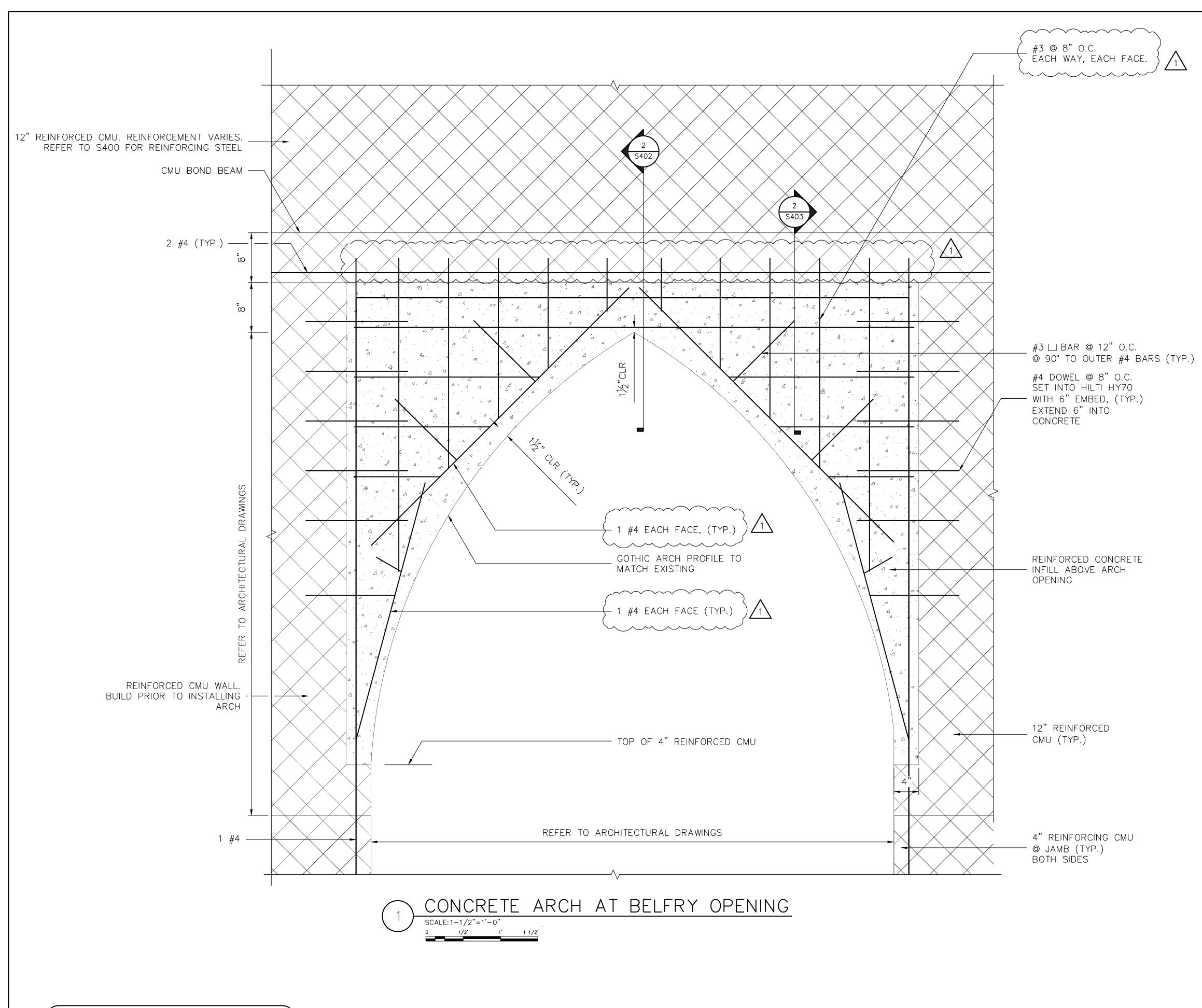
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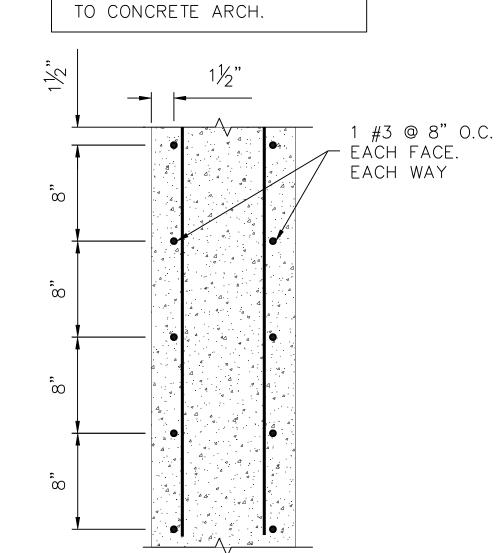
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CONSTRUCTION DOCUMENTS





NOTE: BMU NOT SHOWN FOR CLARITY, REFER TO SPECIFICATIONS FOR BMU TIES



REINFORCEMENT BARS



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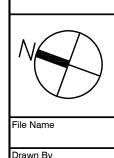
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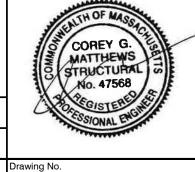
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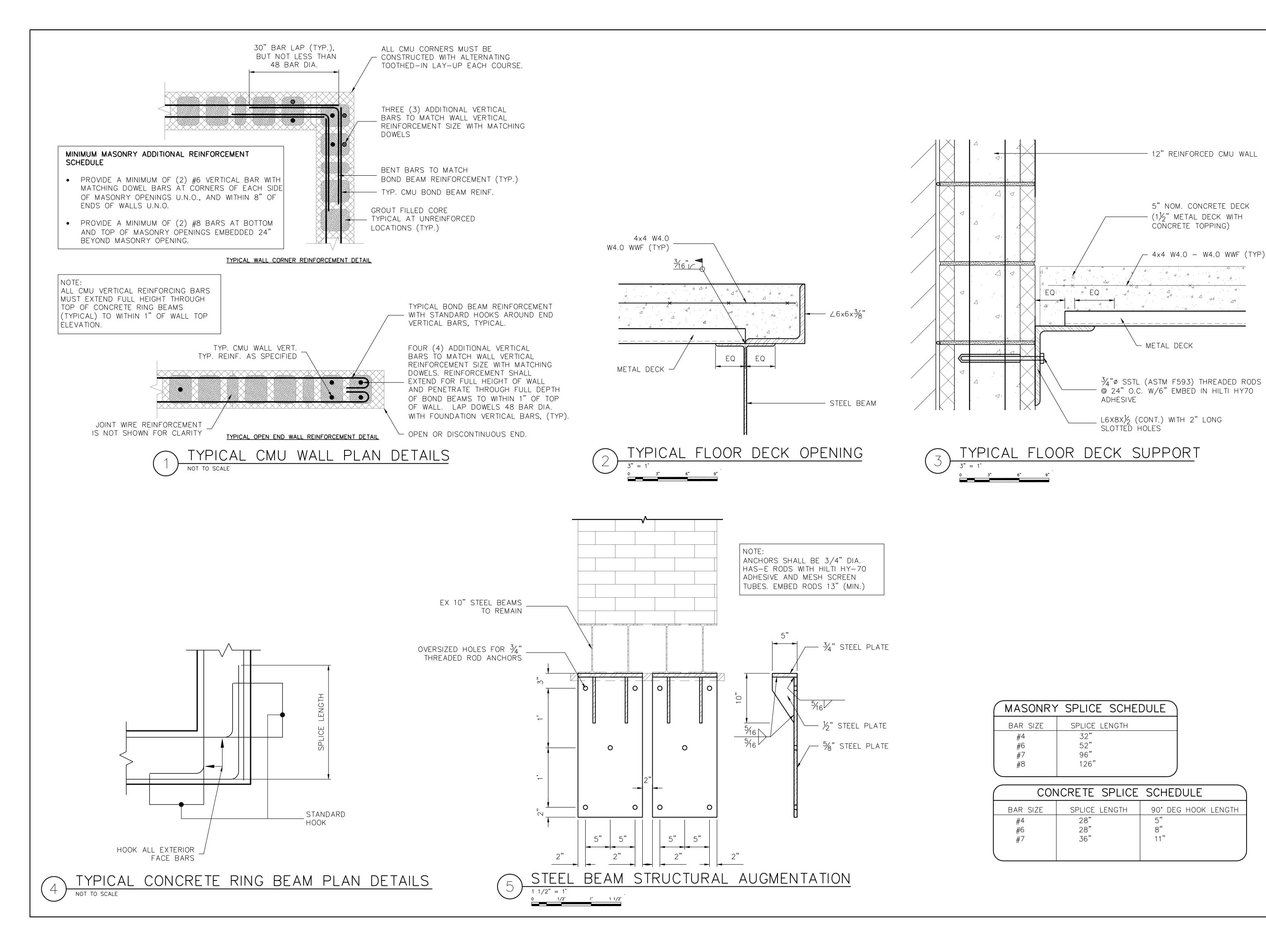


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Date

S403

GENERAL NOTES

- REFER TO S400 FOR BALANCE OF STEEL REINFORCEMENT AND BOND BEAMS.
- REFER TO ARCHITECTURAL DRAWINGS FOR LOUVER CONNECTIONS TO CONCRETE AND







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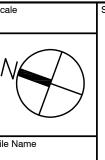
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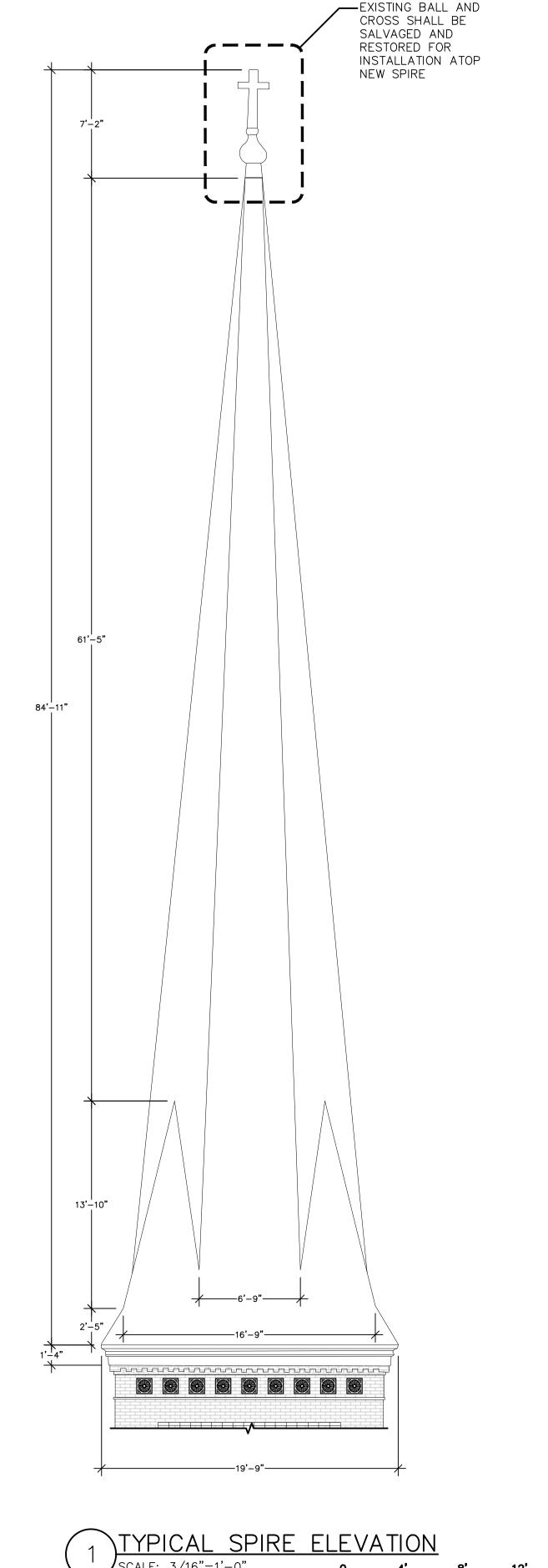
DETAILS



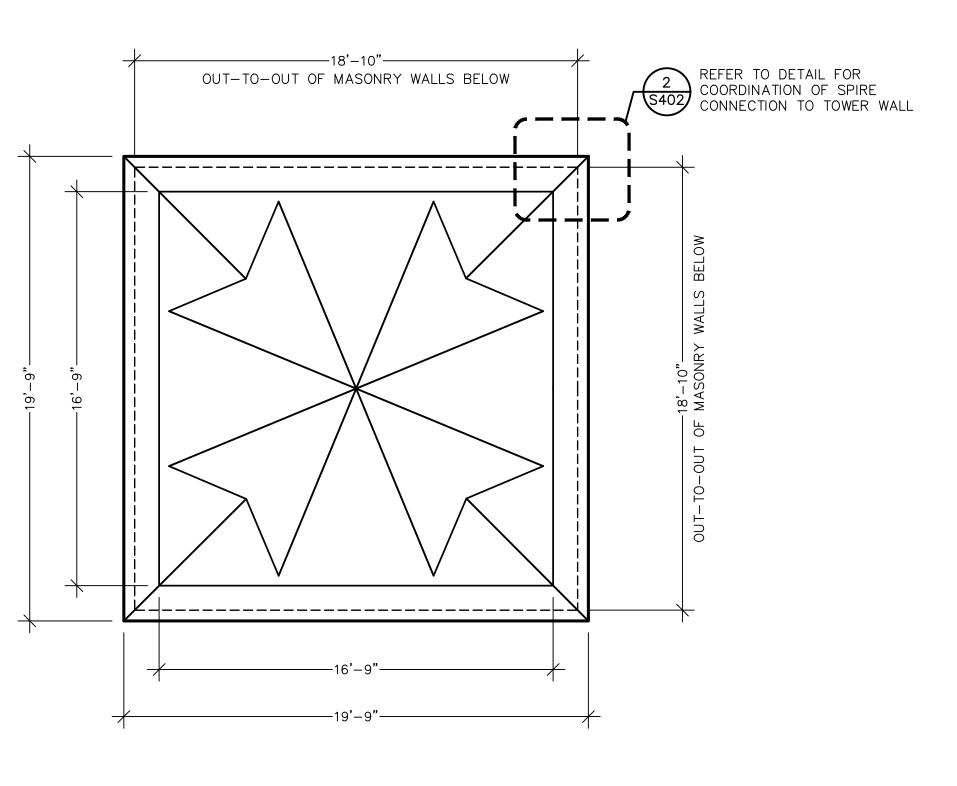


Drawn By
DJZ MLF PDA
Checked By
PDA CM CGM
Job No. 3704
GALE Job No. 832681
Date
9/22/2017

S404



TYP. ALL FOUR (4) ELEVATIONS



2 SPIRE ROOF PLAN SCALE: 1/4"=1'-0" 0 2' 4'

LARGE SPIRE DESIGN CRITERIA

- 1. THE SPIRE STRUCTURAL FRAMING AND ITS COMPONENTS SHALL BE DESIGNED TO CONFORM TO 9TH EDITION OF THE COMMONWEALTH OF MASSACHUSETTS STATE BUILDING CODE (CMR 780).
- 2. THE GEOMETRY OF THE EXISTING SPIRE HAS BEEN GENERATED FROM FIELD MEASUREMENTS, LASER SURVEY, AND PHOTOGRAPHS. THE EXTERIOR FINISH LINES OF THE NEW SPIRE SHALL MATCH THE EXISTING GEOMETRY. NOTIFY THE ENGINEER OF ANY ARCHITECTURAL MODIFICATION OR DIMENSION CHANGES THAT MAY AFFECT THE STRUCTURAL DESIGN OR THE APPEARANCE OF THE NEW SPIRE.
- 3. SPIRE SHALL BE DESIGNED, MANUFACTURED AND DELIVERED TO THE PROJECT SITE AS A COMPLETED SYSTEM WITH ROOFING MATERIALS, PAINT, LOUVERS, AND TRIM FACTORY INSTALLED. SPIRE SHALL BE SEGMENTED FOR SHIPMENT SUCH THAT THE NUMBER OF FIELD SPLICES ARE MINIMIZED. SPIRE MANUFACTURER SHALL PROVIDE TEMPORARY BRACING OF SEGMENTS REQUIRED FOR TRANSPORTATION, HANDLING AND ERECTION.
- . SPIRE SHALL BE EQUIPPED WITH A LIGHTENING PROTECTION SYSTEM THAT CAN BE TIED INTO THE EXISTING COPPER CONDUCTORS AT THE TOP OF THE BELFRY.
- 5. THE SPIRE FRAMING SHALL BE DESIGNED AND DETAILED AS "LIMITED COMBUSTIBLE" IN ACCORDANCE WITH THE NATIONAL FIRE PROTECTION AGENCY (NFPA-13). ALL MATERIALS AND ASSEMBLIES MUST BE TESTED IN ACCORDANCE WITH ASTM-84 OR ANSI/UL 723.
- 6. REFER TO DRAWING EA510'S FOR INFORMATION REGARDING SLATE ROOF ASSEMBLIES TO BE INCORPORATED INTO NEW SPIRE ROOF.
- 7. SPIRE DESIGN AND DOCUMENTATION SHALL INCLUDE THE FOLLOWING:
- LAYOUT DRAWINGS: SCALED DRAWINGS SHALL PROVIDE OVERALL PLAN DIMENSIONS, ELEVATIONS, MEMBER SIZES, MATERIALS, CONNECTIONS, SPLICES, FASTENERS, TEMPORARY AND PERMANENT BRACING, ETC.
- CALCULATIONS: STRUCTURAL DESIGN CALCULATIONS FOR FRAMING MEMBERS, INCLUDING CONNECTIONS, SHALL
 DEMONSTRATE CONFORMANCE WITH DESIGN AND PERFORMANCE CRITERIA. CALCULATIONS SHALL BEAR THE SEAL
 OF A PROFESSIONAL STRUCTURAL ENGINEER, REGISTERED IN THE STATE OF MASSACHUSETTS.
- PRODUCT DATA: PROVIDE MATERIAL DATA SHEETS, MILL CERTIFICATES, SAMPLES (UPON REQUEST), ETC. FOR MATERIALS AND COMPONENTS USED TO MANUFACTURE THE NEW SPIRE.
- 8. ELECTRONIC SUBMISSION OF SHOP DRAWINGS FOR REVIEW AND APPROVAL BY THE ARCHITECT AND/OR ENGINEER IS ACCEPTABLE. REFER TO THE PROJECT SPECIFICATIONS FOR SHOP DRAWING SUBMITTAL REQUIREMENTS.
- 9. SEE PLANS AND ELEVATIONS FOR SPIRE CONFIGURATION AND DIMENSIONS. THE FOLLOWING MINIMUM DESIGN CRITERIA SHALL BE USED FOR PROPORTIONING FRAMING MEMBERS:
- DEFLECTION CRITERIA:

LIVE LOAD DEFLECTION < L/360

TOTAL DEFLECTION < L/240

LATERAL DRIFT < H/200

LOADING CRITERIA:

LIVE LOAD ON INTERNAL PLATFORMS AND STAIRS......100 PSF OR 350 LBS

SEISMIC CRITERIA: (NON-STRUCTURAL COMPONENTS)

RISK CATEGORY = III

SEISMIC DESIGN CATEGORY = C

COMPONENT IMPORTANCE FACTOR, IP = 1.0 SITE CLASS E (UNKNOWN SOIL CONDITIONS)

SPECTRAL ACCELERATION (SHORT PERIOD), SDS = 0.357

COMPONENT AMPLIFICATION FACTOR, $A_P = 2.5$

COMPONENT RESPONSE MODIFICATION FACTOR, $R_p = 1.0$

COMPONENT OPERATING WEIGHT, W_P

• WIND CRITERIA: (ASCE 7-10 - DIRECTIONAL PROCEDURE)

BASIC WIND SPEED, V = 139MPH (ULTIMATE)

EXPOSURE CATEGORY = B

RISK CATEGORY = III (OCCUPANCY GREATER THAN 300)

EXPOSURE COEFFICIENT, K_Z (TABLE 27.3-1)

TOPOGRAPHIC FACTOR COEFFICIENT, $K_{ZT} = 1.0$

DIRECTIONALITY FACTOR, $K_D = 0.95$

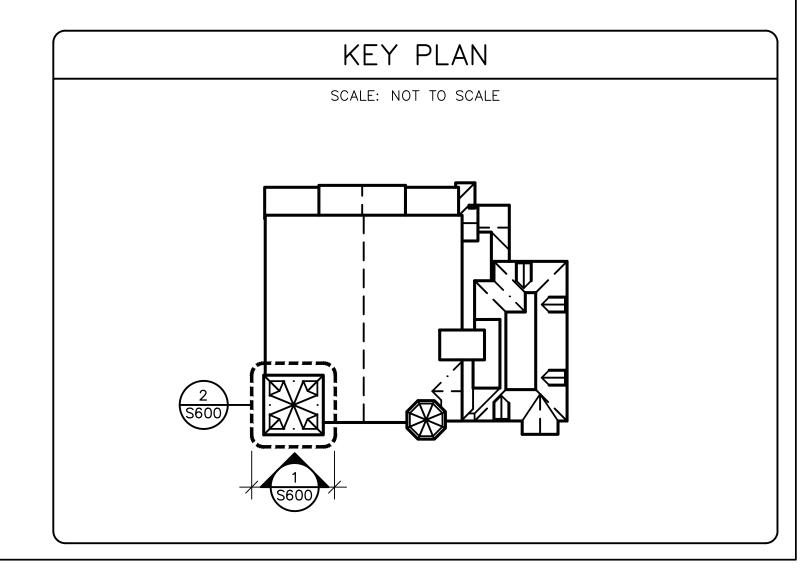
GUST FACTOR, G = 0.85

VELOCITY PRESSURE, $Q_Z = 0.00256 * K_Z * K_{ZT} * K_D * V_2$ (PSF)

HEIGHT OF APPLIED WIND, Z = 113 FEET (APPROX.)

FORCE COEFFICIENT, $C_F(TABLE 27.4-1)$

DESIGN WIND FORCE, $F = Q_Z * G * C_F * A_F$ (LBS)





54 Canal Street 2nd Floor Boston, MA 02114 617-267-6408

Fax 617-267-1990

ARCHITECTS



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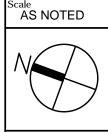
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PHASE 1A CONSTRUCTION DOCUMENTS

SPIRE DESIGN CRITERIA



le Name

Drawn By
KPB EWM SMF
Checked By
MDF CM CGM

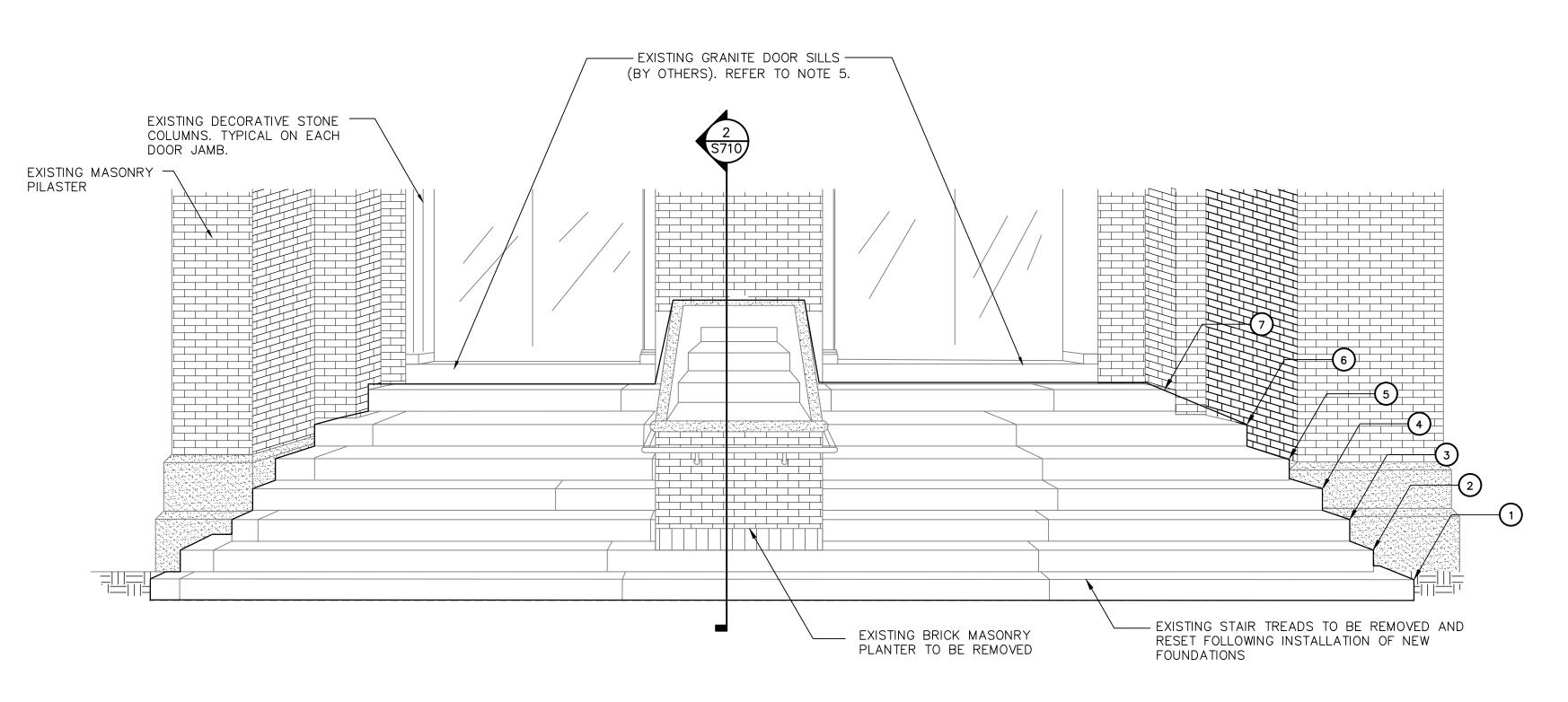
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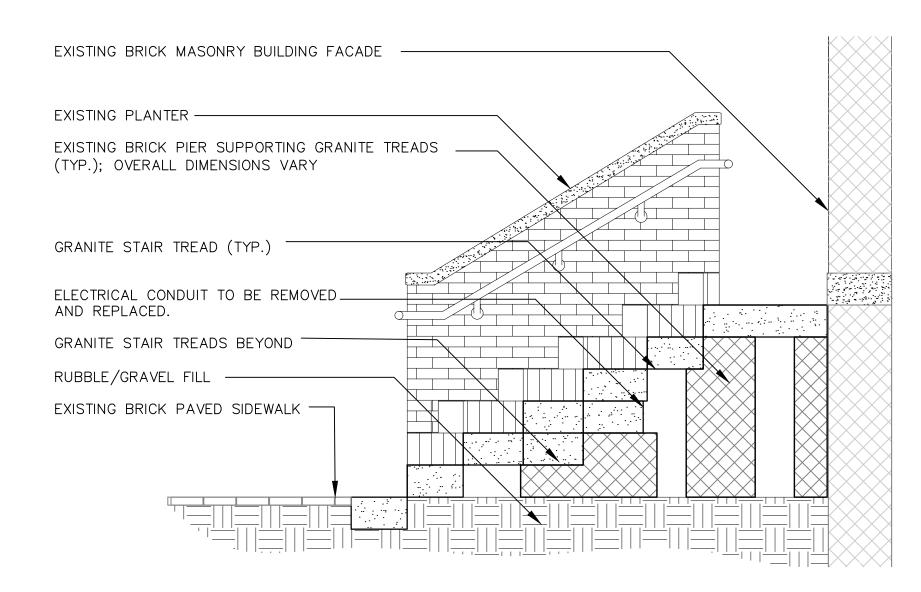
Job No. 3704

GALE Job No. 832681

Date



1 EXISTING CONDITIONS / DEMOLITION GRANITE STAIRS ELEVATION SCALE: N.T.S.



EXISTING GRANITE STAIR SECTION

SCALE: 1/2"=1'-0"

1 2' 4'

GENERAL NOTES

- 1. THE INFORMATION SHOWN ON THIS PLAN HAS BEEN COMPILED FROM VARIOUS SOURCES AND INDICATES REPRESENTATIVE DEFECTS AND CONDITIONS. THE CONTRACTOR SHALL VERIFY ALL CONDITIONS, DIMENSIONS, AND QUANTITIES IN THE FIELD.
- 2. ANY CONDITION FOUND NOT DETAILED SHALL BE CONSTRUCTED IN A MANNER SIMILAR TO THAT OF THE TYPICAL DETAILS.
 IF CONDITIONS EXIST THAT PRECLUDE CONSTRUCTION SIMILAR TO TYPICAL DETAILS, THE CONTRACTOR SHALL INFORM THE ENGINEER PRIOR TO PROCEEDING WITH CONSTRUCTION.
- 3. ALL DIMENSIONS AND LOCATIONS OF EXISTING FOUNDATION ELEMENTS ARE BASED UPON LIMITED VISUAL OBSERVATIONS AND ARE APPROXIMATE.
- 4. THE CONTRACTOR IS TO PERFORM AN INSTRUMENT FIELD SURVEY TO VERIFY EACH STAIR TREAD ELEVATION PRIOR TO DEMOLITION.
- 5. TO AVOID REMOVAL AND SHORING OF ADJACENT BRICK MASONRY AND ASSOCIATED STONE ELEMENTS AROUND THE ENTRY DOORS, THE UPPERMOST GRANITE TREADS MAY REMAIN IN PLACE WHILE EXISTING SUPPORTS ARE REPLACED WITH NEW PIER FOUNDATIONS. THE CONTRACTOR MUST PROVIDE ADEQUATE TEMPORARY SHORING AS REQUIRED.



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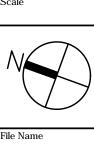
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PHASE 1A CONSTRUCTION DOCUMENTS

PROPOSED DEMOLITION
AND EXISTING STAIR
SECTION



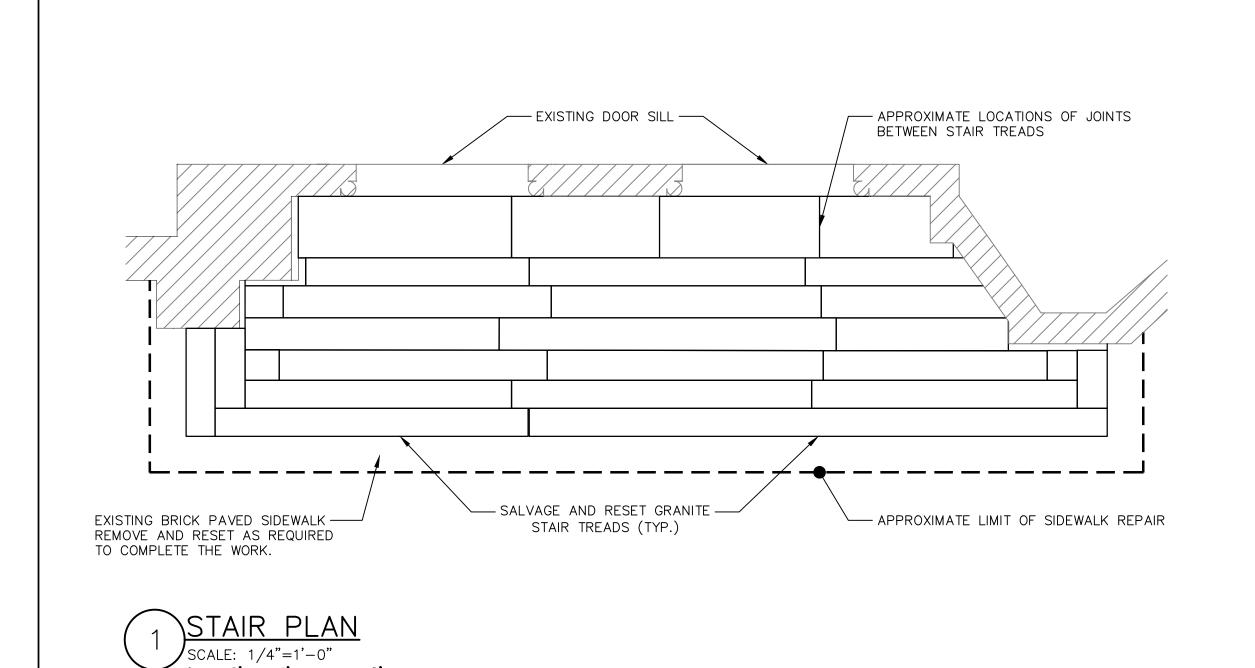
COREY G.
MATTHEWS
STRUCTURAL
No. 47568

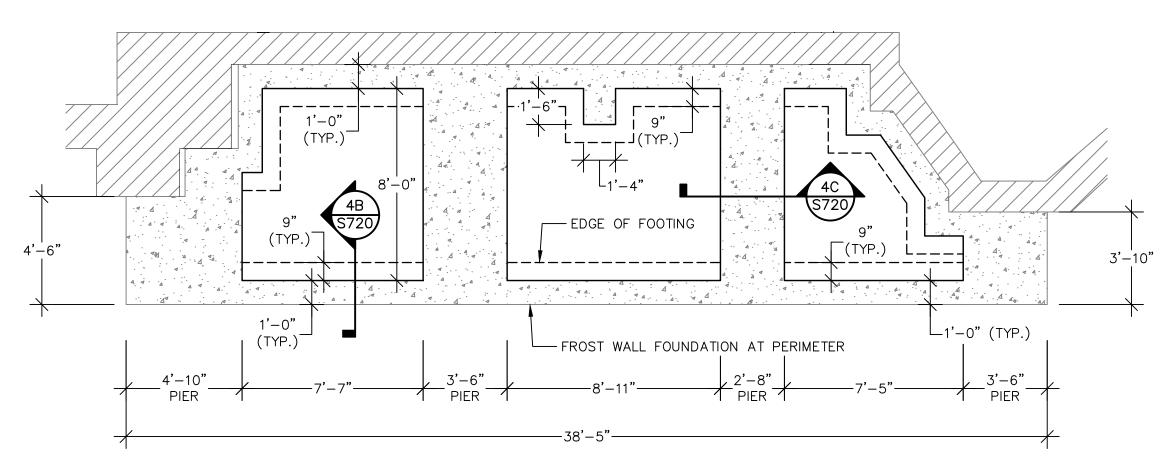
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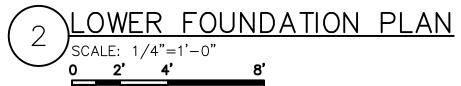
KPB EWM SMF
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MDF CM CGM
Job No. 3704
GALE Job No. 832681

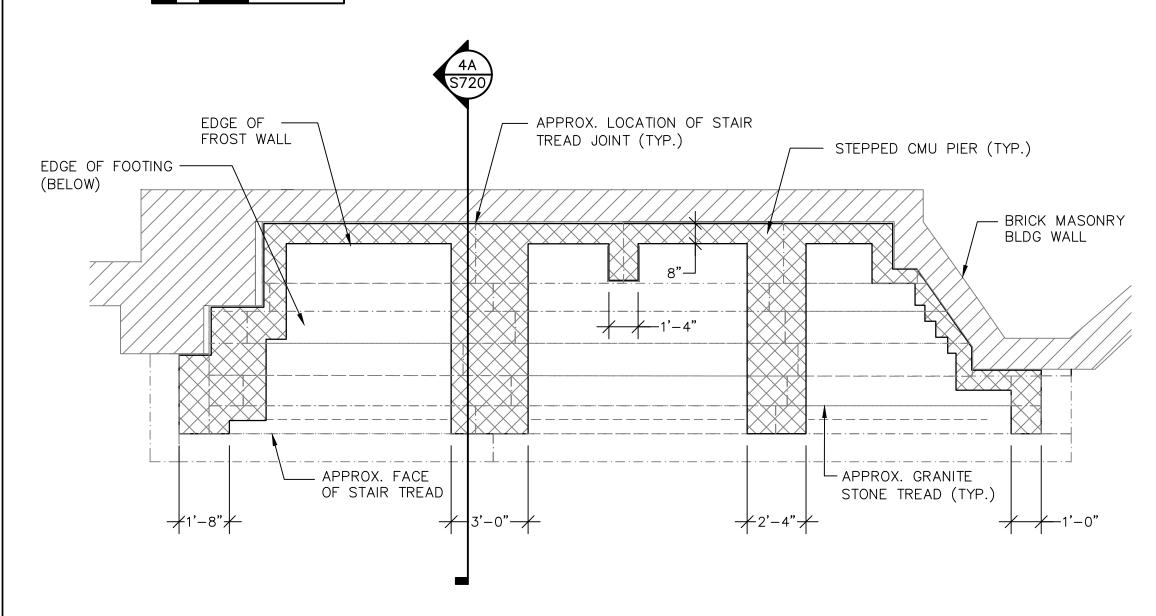
9/22/2017

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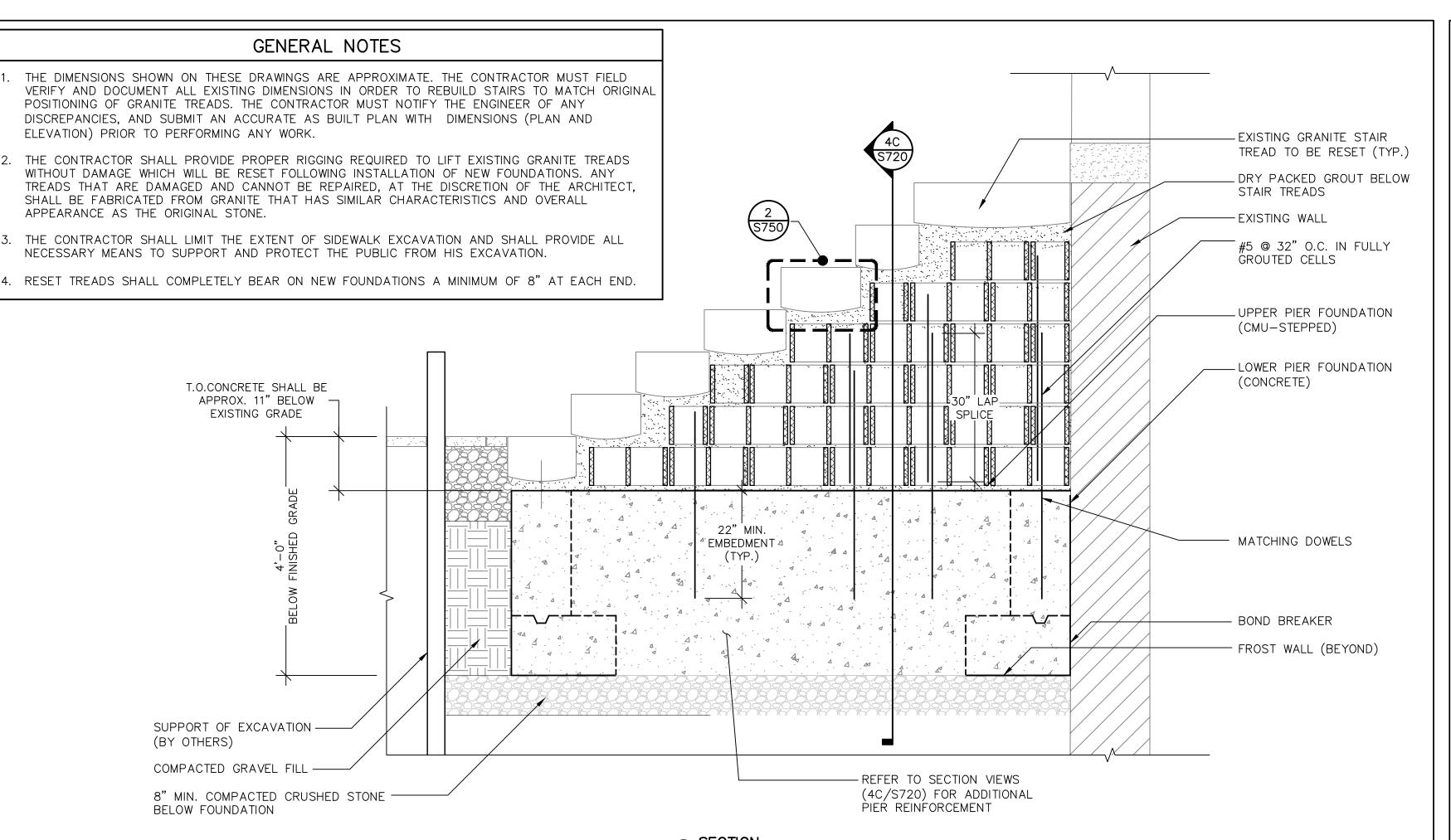


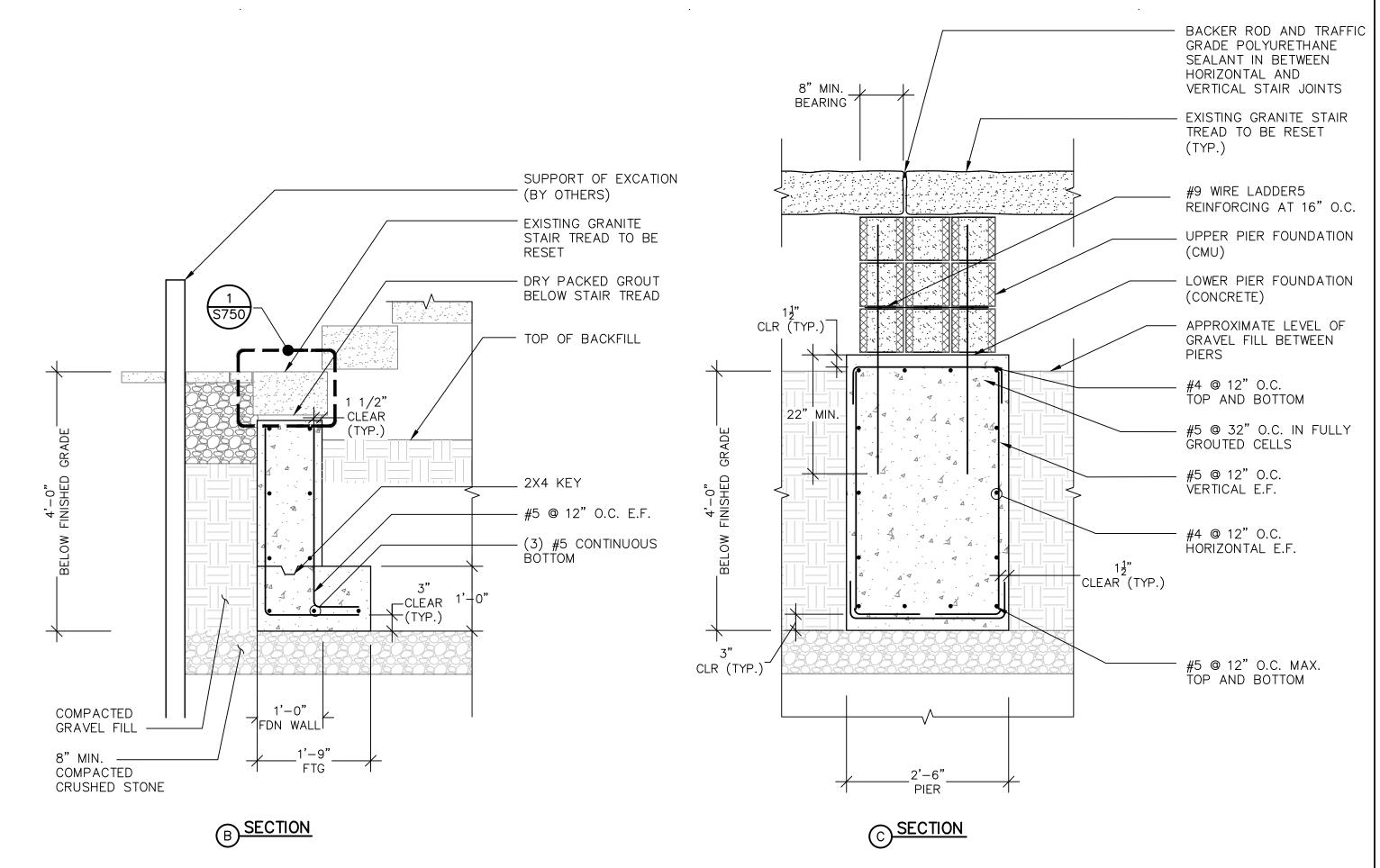






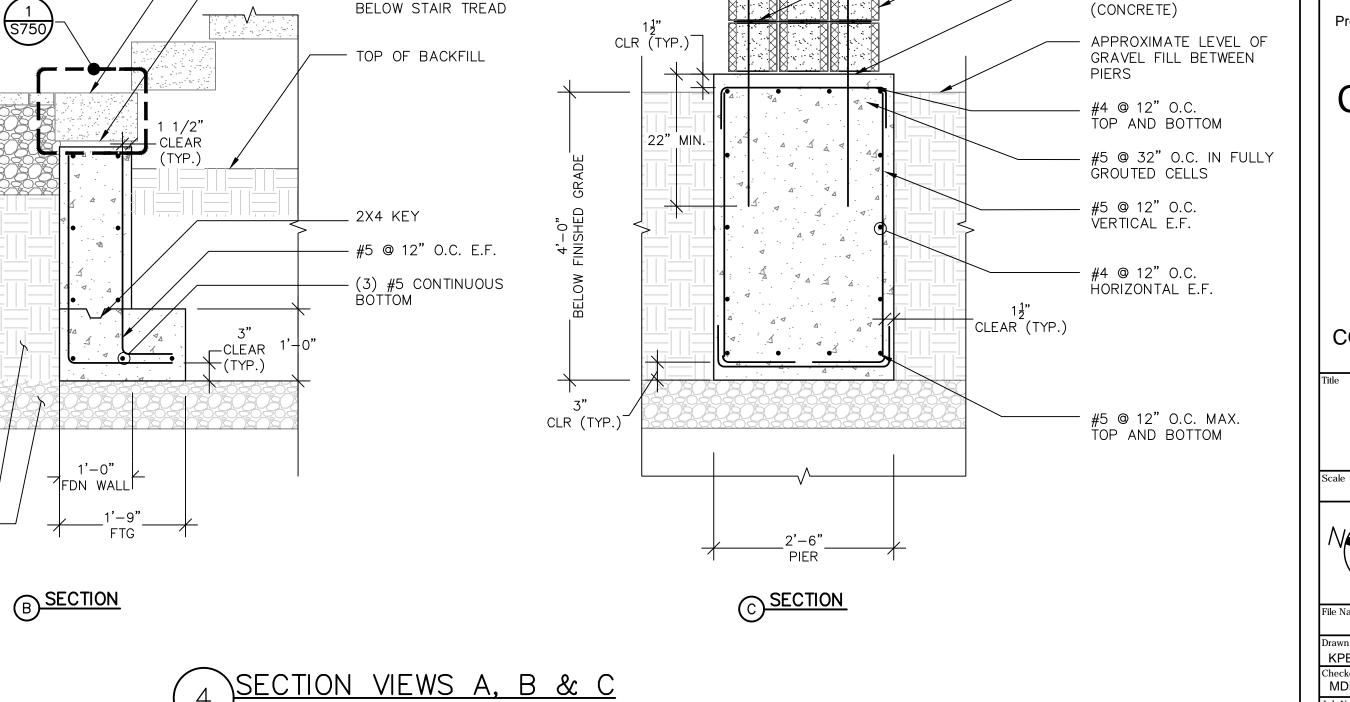
JPPER FOUNDATION PLAN

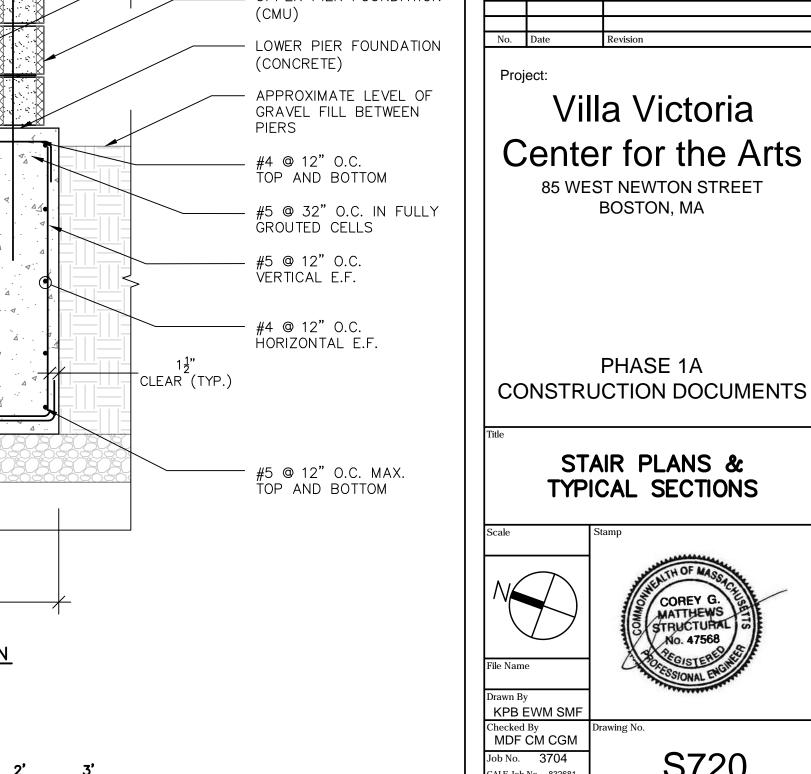




SCALE: 3/4"=1'-0"

(ALL ITEMS ARE NEW UNLESS DESIGNATED AS EXISTING)







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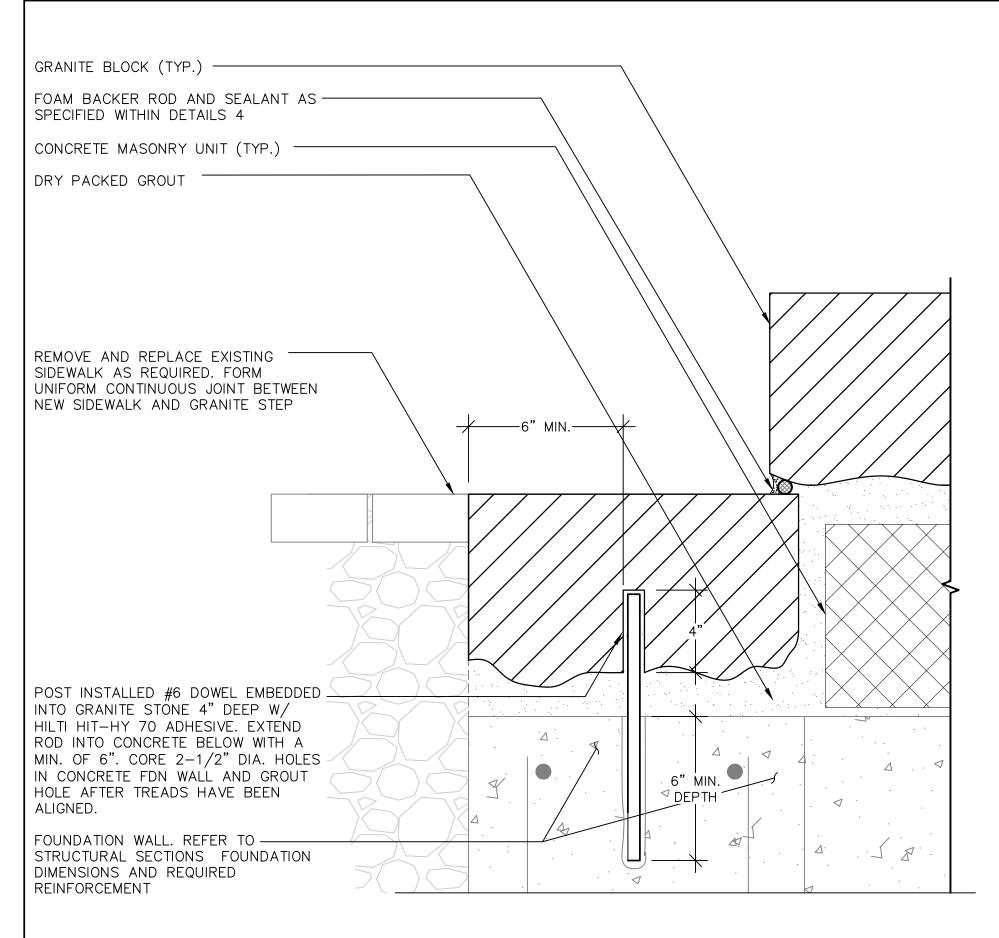
STAIR PLANS &

TYPICAL SECTIONS

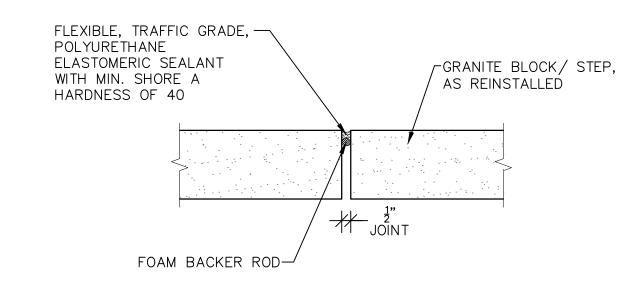
GALE Job No. 832681 Date

9/22/2017

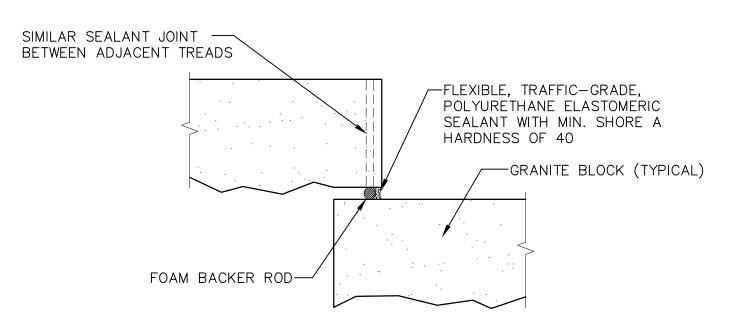
S720



GRANITE BLOCK (TYP.) ---FOAM BACKER ROD AND SEALANT AS SPECIFIED WITHIN DETAILS 4 CONCRETE MASONRY UNIT PIER (TYP.) DRY PACKED GROUT ----—6" МIN.—— POST INSTALLED #6 DOWEL EMBEDDED INTO GRANITE STÖNE 4" DEEP W/ HILTI HIT-HY 70 ADHESIVE. EXTEND ROD INTO CONCRETE BELOW WITH A MIN. OF 6". GROUT CELLS AND DRY-PACK STONES AFTER TREADS HAVE BEEN ALIGNED. OFFSET AND STAGGER NEW HORIZONTAL JOINTS FOR GRANITE STAIR TREADS TO MATCH EXISTING JOINT LOCATION. PITCH GRANITE STEPS $\frac{1}{6}$ " OUTWARDS (TYP.). 6"MIN. ".DEPTH."



HORIZONTAL SEALANT JOINT



VERTICAL SEALANT JOINT

TYP. BASE STONE ANCHOR PIN DETAIL SCALE: 3"=1'-0" (ALL ITEMS ARE NEW UNLESS DESIGNATED AS EXISTING)

2 TYP. STONE TREAD ANCHOR PIN DETAIL

SCALE: 3"=1'-0"
(ALL ITEMS ARE NEW UNLESS DESIGNATED AS EXISTING)

TYP. STONE JOINT SEALANT DETAIL

SCALE: 3"=1'-0"
(ALL ITEMS ARE NEW UNLESS DESIGNATED AS EXISTING)



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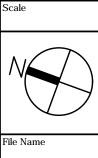
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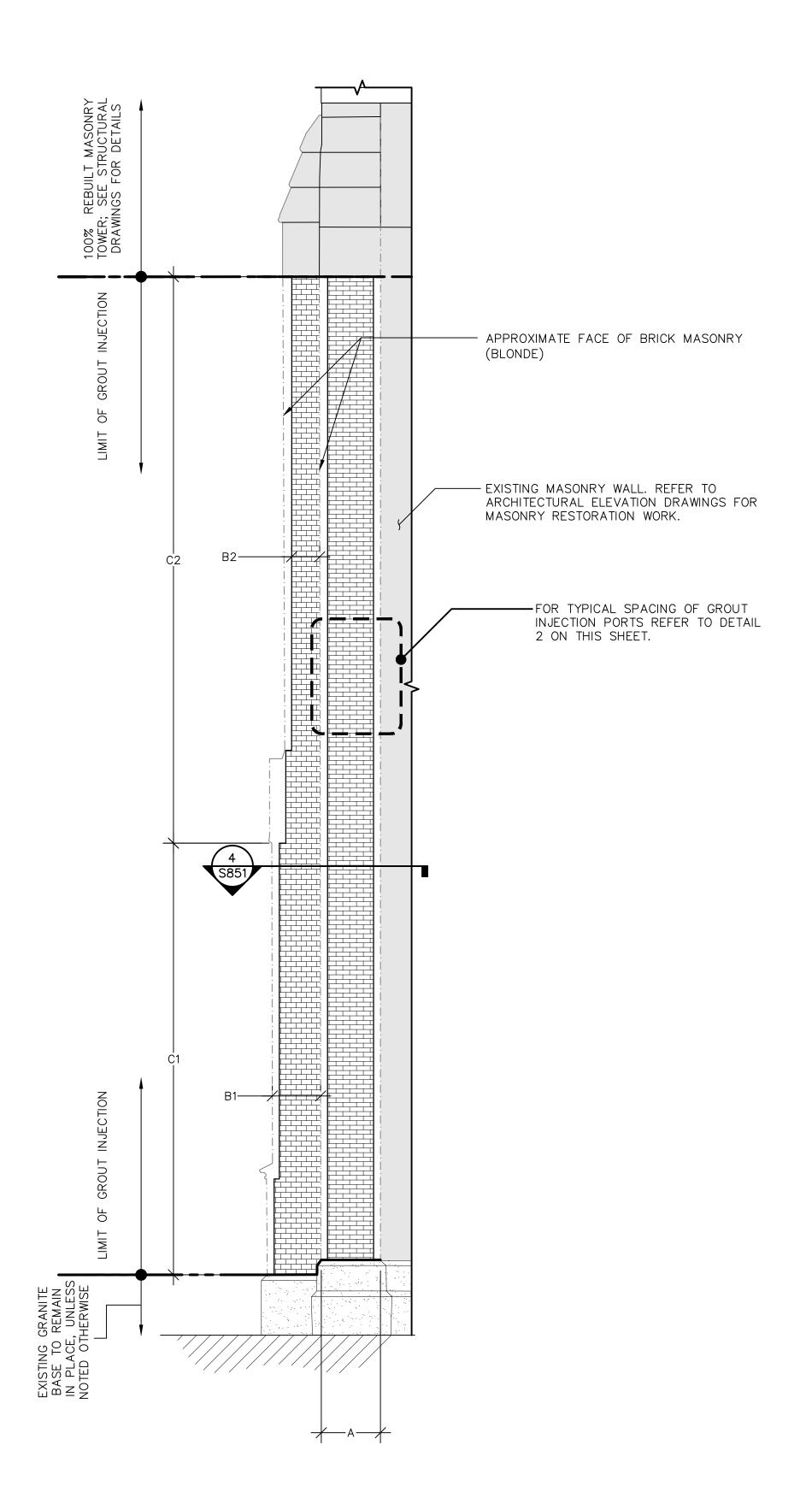
TYPICAL STAIR DETAILS



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MDF CM CGM
Job No. 3704

S750

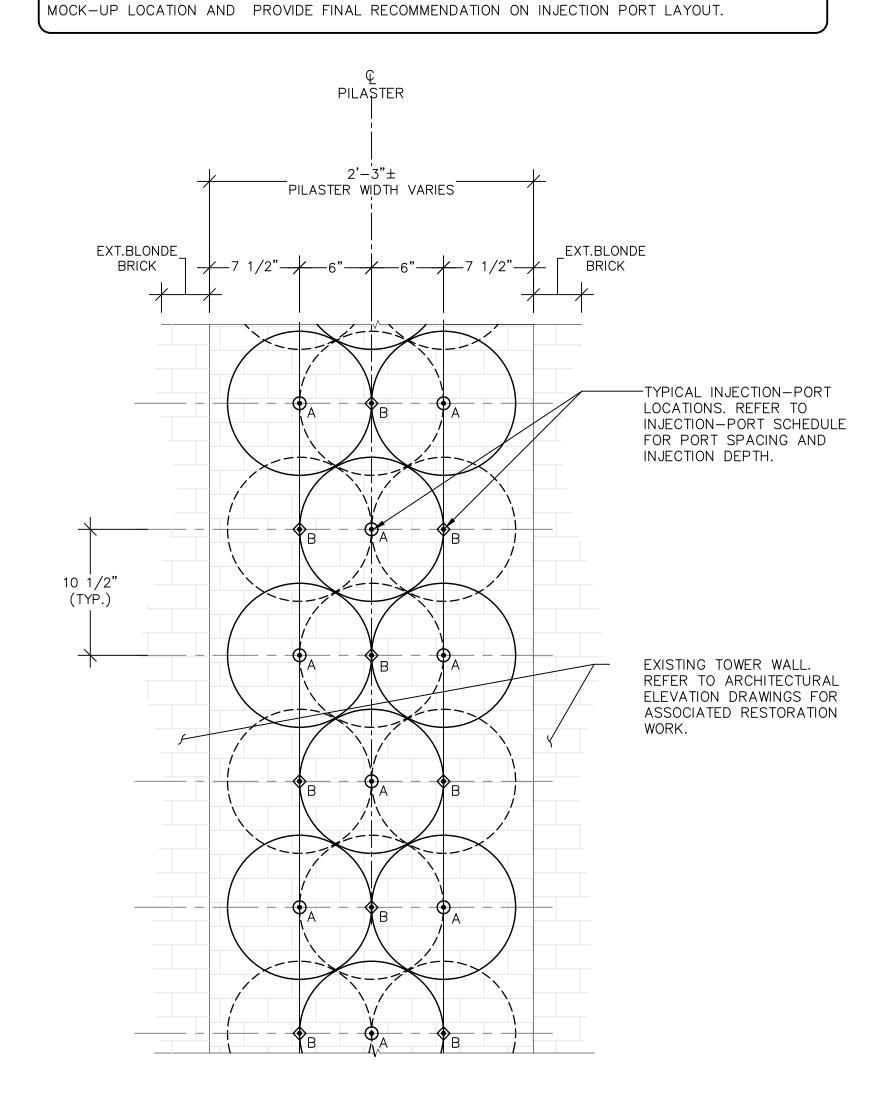
NOTE: DIMENSIONS PROVIDED ON THIS DETAIL ARE APPROXIMATE AND SHALL BE USED TO ESTIMATE THE QUANTITY OF RED BRICK BEHIND THE BLONDE BRICK EXTERIOR COURSE. REFER TO THE EXISTING BRICK MASONRY PILASTER SCHEDULE FOR THE INDICATED LETTERED DIMENSIONS.



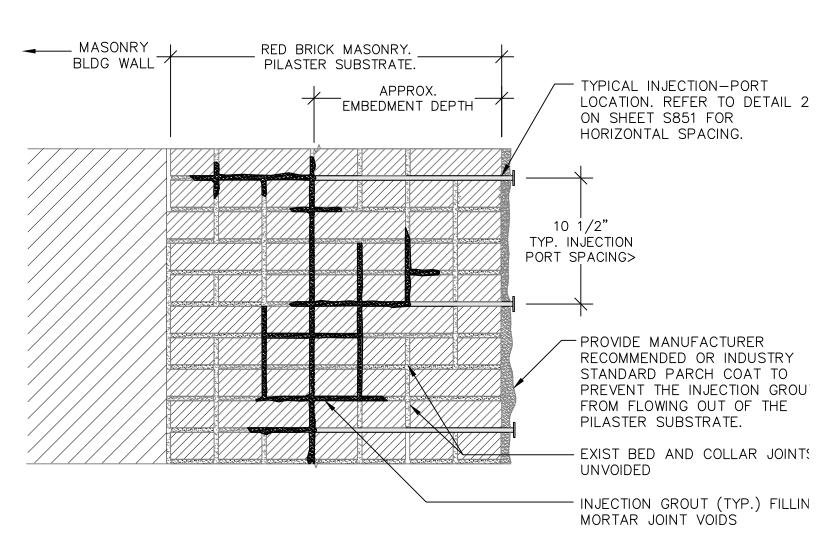
INJECTION-PORT SCHEDULE

 $oldsymbol{\Theta}_{\Lambda}$ port 'a' 16" deep (extend to 4th collar joint) ♠ PORT 'B' 8" DEEP (EXTEND TO 2ND COLLAR JOINT)

NOTE: DEPENDING ON THE ABILITY OF THE GROUT TO FLOW LATERALLY THROUGH THE DEPTH OF THE PILASTER VIA OPEN HEAD JOINTS/ CRACKS, PORT 'B' MAY NOT BE NEEDED OR REQUIRE LESS NUMBERS. ENGINEER OF RECORD IS TO REVIEW AND APPROVE BY THE CONTRACTOR PREPARED



TYP. GROUT INJECTION—PORT LAYOUT ELEVATION



TYP. GROUT INJECTION PILASTER SECTION

EXISTING BRICK MASONRY PILASTER SCHEDULE (*)

PILASTER #	WIDTH (A)	DEPTH (B1)	DEPTH (B2)	HEIGHT (C1)	HEIGHT (C2)
P1	3'-2" ±	2'-5" ±	1'-10" ±	16'-1" ±	32'-6" ±
P2	0'-8" ± 2'-10" ±(*)	2'-7" ±	1'-6" ±	20'-1" ±	32'-6" ±
Р3	2'-10" ±	2'-7" ±	1'-6" ±	20'-1" ±	32'-6" ±
P4	2'-10" ±	2'-8" ±	1'-8" ±	20'-1" ±	32'-6" ±
P5	2'-10" ±	2'-7" ±	1'-10" ±	20'-1" ±	32'-6" ±
P6	3'-2" ±	2'-3" ±	1'-6" ±	20'-1" ±	32'-6" ±
P7	2'-5" ±	2'-10" ±	2'-0" ±	7'-0" ±	9'-6" ±

- * PILASTER WIDTH VARIES THROUGHOUT THE PILASTER HEIGHT. PILASTER PARTIALLY EMBEDDED AND TERMINATED AT BLDG ROOF.
- ** INDICATED MEASUREMENTS FOR BLOND BRICK WERE TAKEN ABOVE THE GRANITE PILASTER BASE. DIMENSIONS VARY BASED ON PILASTER ELEVATION. CONTRACTOR IS TO CATALOGUE AND VERIFY ALL PILASTER DIMENSION PRIOR TO COMMENCING WITH THE DEMOLITION.



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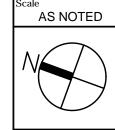
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PHASE 1A CONSTRUCTION DOCUMENTS

PILASTER GROUT-INJECTION SECTION & DETAILS



EXISTING BUILDING WALL/

TOWER CORNER

- EXISTING INNER PILASTER SUBSTRATE

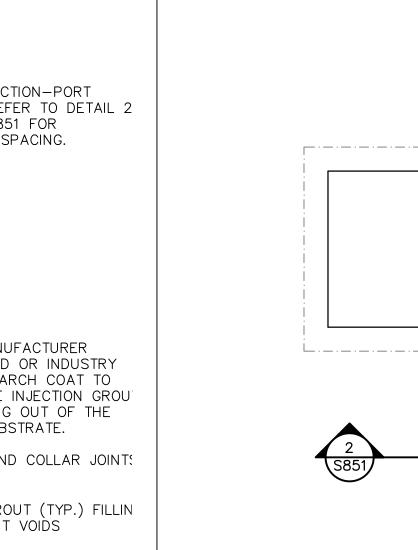
- REMOVED & REBUILT OUTER WYTHE

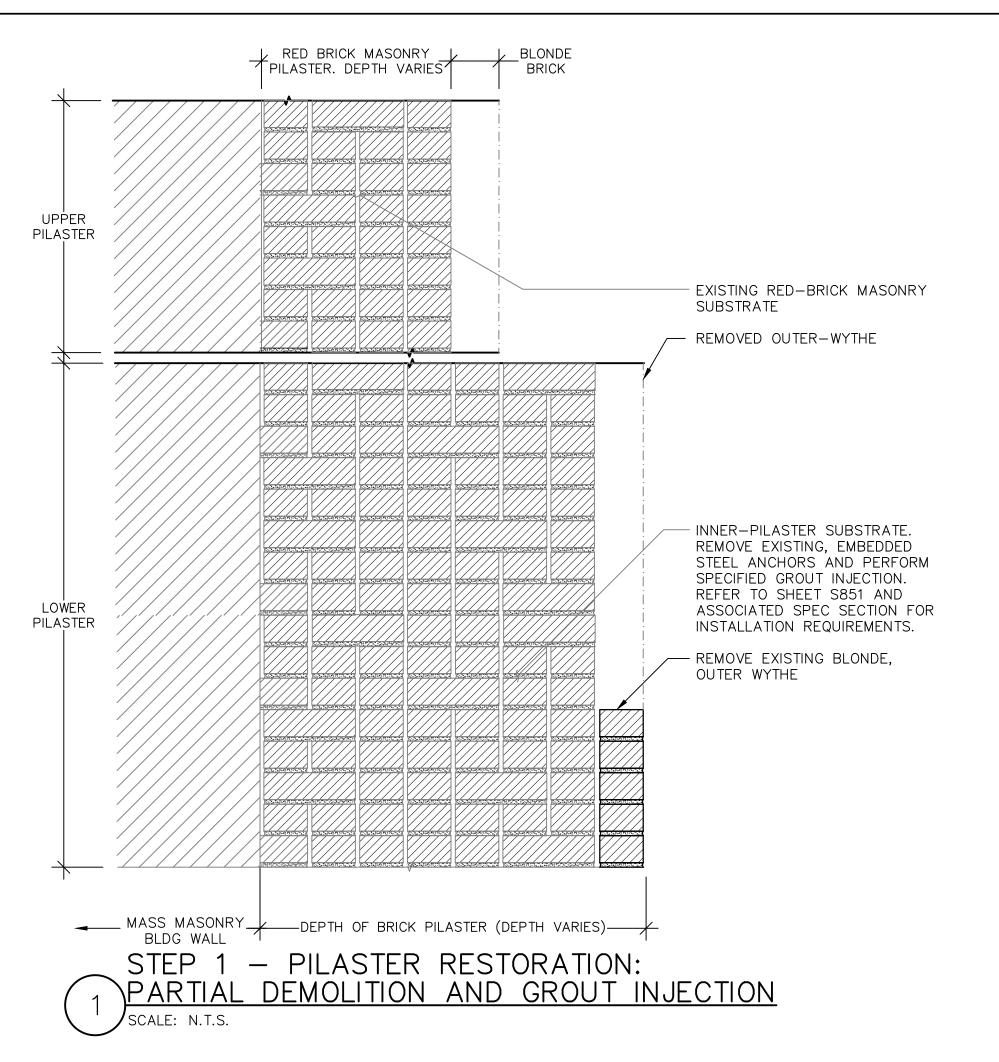
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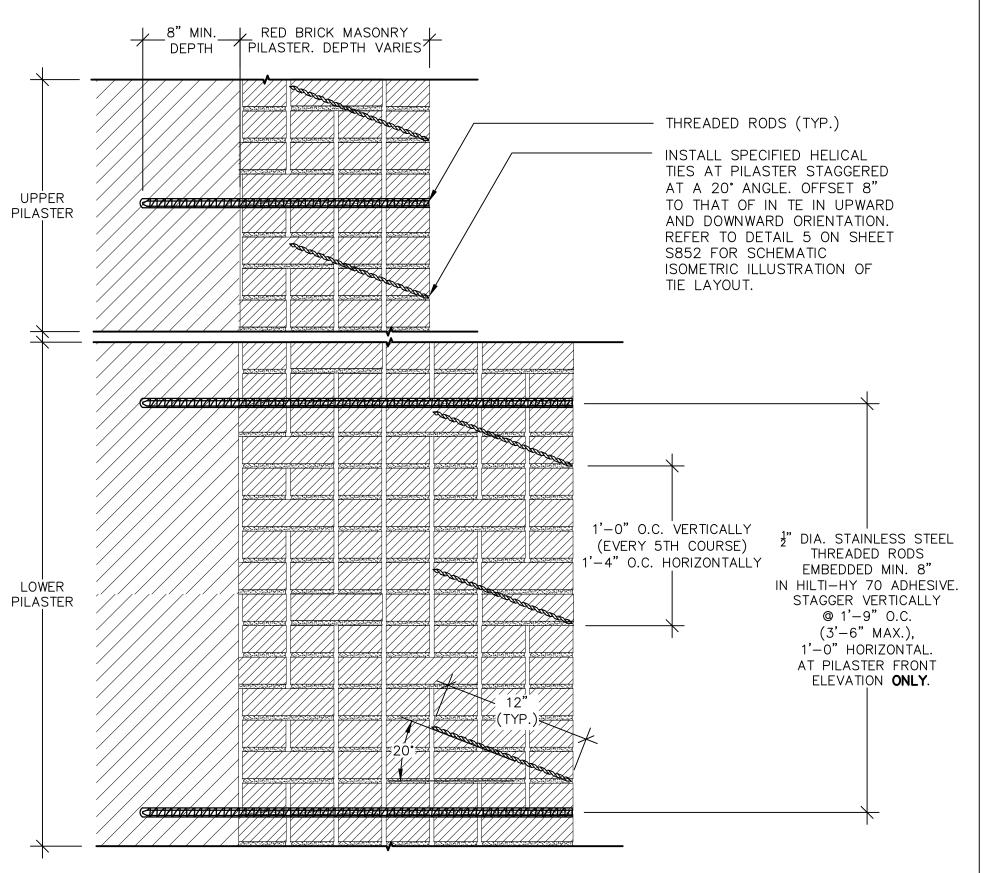
GALE Job No. 832681

Date

S851 9/22/2017







STEP 2 - PILASTER RESTORATION: \THREADED ROD & HELICAL-TIE INSTALLATION SCALE: N.T.S.

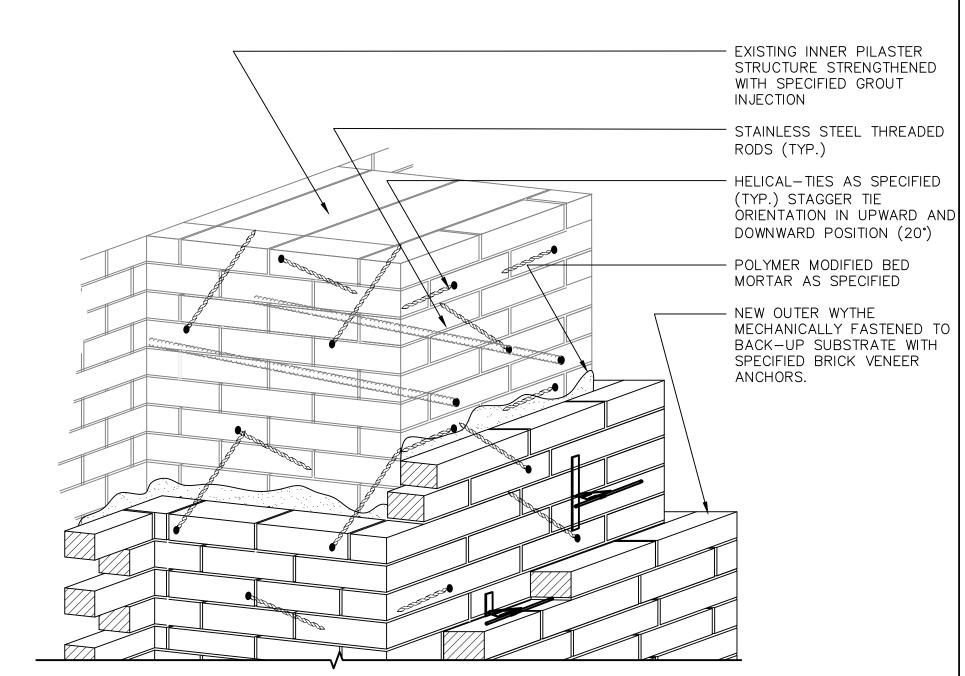
2"MIN.-

NEW STAINLESS STEEL THREADED RODS (TYP.) - NEW SPECIFIED HELI-TIES (TYP.) GROUTED, INNER PILASTER STRUCTURE PILASTER - CLEAN EXTERIOR FACE OF INNER WYTHE FREE OF LOOSE MATERIAL AND OTHER POTENTIAL BOND-BREAKER SPECIFIED POLYMER MODIFIED VENEER MORTAR. PROVIDE %" CONTINUOUS THICK LAYER FREE OF VOIDS ON SURFACE PREPARED INNER WYTHE WALL EXTERIOR BRICK WYTHE MUST BE IN FULL CONTACT W/ WET MORTAR. NEW SPECIFIED HOHMANN & BARNARD BRICK VENEER ANCHORS @ 16" O.C. SECURE WITH 1" DIA. STAINLESS STEEL TAPCON SCREWS. EMBED SCREWS MIN. 13" INTO BRICK UNIT. LOWER PILASTER - PROVIDE BAG-MIX TYPE N MASONRY MORTAR (SPECIFIED) AT ALL BRICK JOINTS AND FINISH PER ARCHITECTS SPECIFICATIONS - REBUILT BLONDE BRICK OUTER COURSE

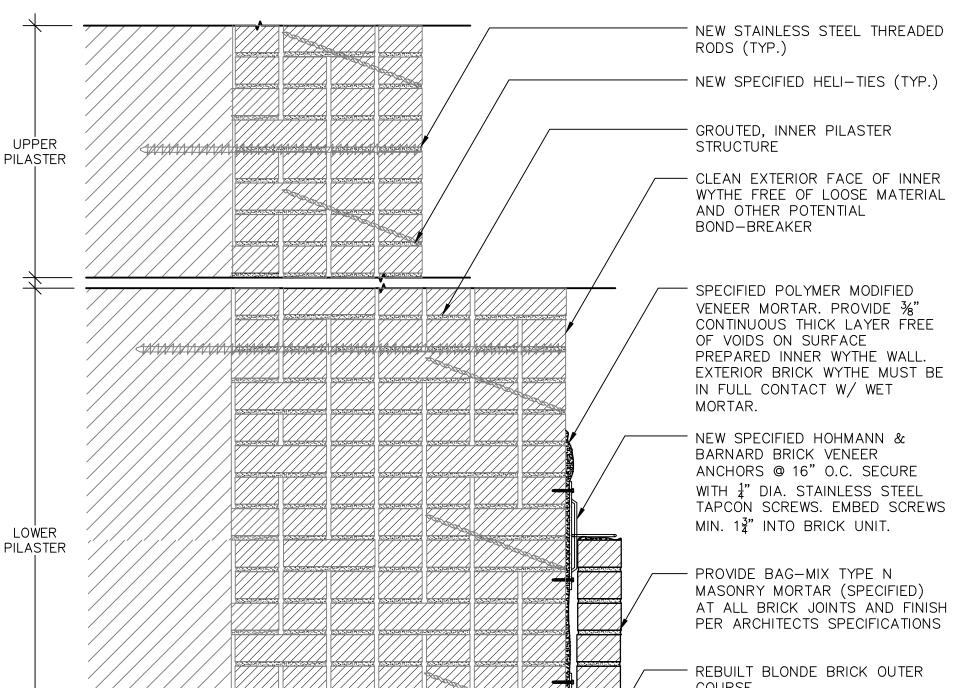
SCALE: N.T.S.

GENERAL PILASTER STRENGTHENING NOTES

- CONTRACTOR MUST PROVIDE TEMPORARY SHORING FOR ALL REPAIR PHASES AS REQUIRED
- HELICAL AND BRICK VENEER TIE ANCHOR LAYOUT MAY VARY THROUGHOUT THE PILASTERS. CONTRACTOR IS TO ADJUST NUMBER AND POSITION OF ANCHORS TO PROVIDE THE SPECIFIED MINIMUM CLEARANCES.



5 FINAL VIEW OF TIE AND PILASTER ANCHOR INSTALLATION



STEP 3 - PILASTER RESTORATION:

BRICK-TIE INSTALLATION & OUTER WYTHE REBUILT

DHK

ARCHITECTS

54 Canal Street

2nd Floor

Boston, MA 02114

617-267-6408

Fax 617-267-1990

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PILASTER STRENGTHENING & **REBUILT**

AS NOTED

KPB EWM SMF Checked By MDF CM CGM GALE Job No. 832681

9/22/2017

S852

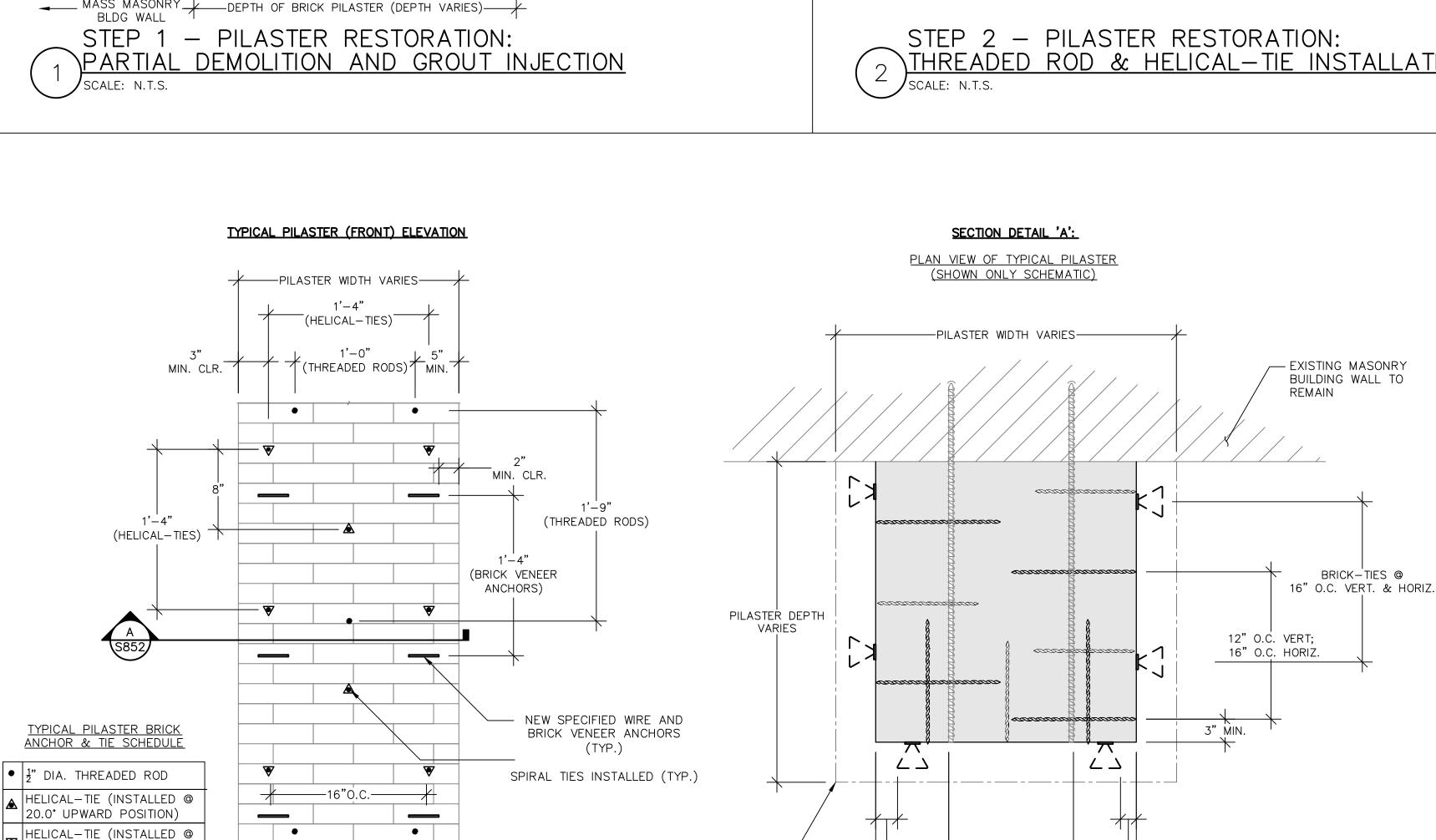
TYPICAL TIE, & PILASTER, ANCHOR LAYOUT

SCALE: 1-1/2"=1'-0"

20.0° DOWNWARD POSITION)

BRICK VENEER ANCHORS

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NEW PLACED OUTER WYTHE

VENEER